



CONTRACT DOCUMENTS & TECHNICAL SPECIFICATIONS ADDENDUM NO. 2

Auburn Heights Preserve Paper Mill Bridge Contract No. 2015-NVF-100

ADDENDUM NO. 2

Project Name: Auburn Heights Preserve

Paper Mill Bridge

Contract No.: 2015-NVF-100

Date of Issue: August 10, 2017

Notice No. 2: Attach this addendum to the Project Manual for this project. It modifies and becomes part of the

Bidding Documents. Work or material not specifically mentioned herein is to be as described in

the main body of the specifications and as shown on the drawings.

Bids Due: August 17, 2017, 1:00 PM

"DNREC CONTRACT NO. 2015-NVF-100 SEALED BID – DO NOT OPEN".

Dept. of Natural Resources & Environmental Control

Division of Parks and Recreation Office of Design and Development 89 Kings Highway, Dover DE 19901

Attn: Cindy A. Todd, RLA. Phone Number: 302-739-9210

QUESTIONS:

1. Is an Affidavit of Employee Drug Testing Program" required to be submitted with the bid for the prime contractor and all listed subcontractors? If so, will the Department provide the correct form?

Response: An Affidavit of Employee Drug Testing Program" is required to be submitted with the bid for the prime contractor and all listed subcontractors? The Affidavit has been added to the Bid Form and is included in this Addendum.

2. Demo Plan Sheet C-11 calls out notes 3 & 5, there is no note 5.

Response: Sheet C-11, the call out referencing notes 3 and 5 has been revised and the revised sheet is included in this Addendum.

3. What fence items are included in the fence allowance? Does it include the split rail fence in Alternate No. 1?

Response: The fence items included in the allowance will be determined by the owner during construction. The split rail fence shown in Add/Alternate No. 1 is not part of this allowance and shall be included in the lump sum bid for Add/Alternate No. 1.

4. Sheet C-1, Note 13, specifies no Portland cement fragments in the base material. Will DelDOT approved crushed concrete be approved as a base material for the project?

Response: DelDOT approved crushed concrete shall be acceptable as a base material for the project.

5. Will the Department consider adding an allowance for the repair work that may be need to the existing HI Parcel Trail after it is utilized as an access road.

Response: The Department has added Allowance No. 5: Trail Repairs, totaling \$20,000 to the base bid. This allowance will be used as directed by the Owner for repairs to the HI Parcel Trail after it has been utilized as an access road. The Allowance specification and the bid forms have been updated and are included in this Addendum.

6. Who will be responsible for the Bog coordination and inspection? Who will be responsible for the compensation?

Response: The contractor will be responsible to coordinate with the Bog Turtle surveyor as detailed on Sheet E-1, "Working in Waterways". The Bog Turtle inspection will be performed by the Bog Turtle surveyor. The Bog Turtle surveyor will be under contract to the Department.

7. Sheet 15, Detail 10C-15, not sure what is going on here?

Response: The intent of the detail is to show the scoring pattern for the area. Clarifications were made to Sheet 15, Detail 10C-15 and issued in Addendum No. 1.

8. What is the overall scope that will be expected for the parking facility on Benge Road? Does the scope of work include repairs to the historic wall?

Response: Repair to the historic wall is not included in the scope of work for the parking lot on Benge Road.

9. We need a typical section for Sta. 300+43 to 301+12 pertaining to Alternate No. 1.

Response: The geometric layout for this area is shown on Sheet C-9. Notes was added to clarify the pavement and sidewalk section for this area. The revised sheet is included in this Addendum. 10. Will the Department require a field office (Section 015000)?

Response: The contractor shall include a field office on the HI Parcel. The exact location of the field office shall be coordinated with the Owner in the field. The field office shall include a minimum of one desk and chair each for the Owner and the inspection engineer. All costs associated with the field office shall be included in the Base Bid.

11. The pre-bid meeting minutes indicate that the subcontractor list will be "reduced by eliminating the subs for the allowances." Will the bid forms be reissued with the correct sub listing?

Response: The subcontractor list has been revised and is included in Addendum No. 2.

12. The pre-bid meeting minutes indicate that railing (from sheet P4) will be depicted on a new plan sheet. Will the plan sheet be issued soon?

Response: New plan sheets P-5 and P-6 have been created to depict the railing locations and are included in this Addendum.

13. Section 600500 requires "At the time of the bid, the Contractor shall identify and submit qualifications for the Contractor's Engineer" Will the Department be providing a format or form for this submission?

Response: The Contractor's Engineer was removed from the sub-contractors list and is part of the allowance for the Deck Over Structure. Once the contract is awarded the contractor will be required to submit documentation that the Contractor's Engineer is a registered professional engineer in the state of Delaware and evidence of successfully completing three projects of similar size and scope within the last five years. Documentation shall include name of project, Owner, owner contact information and date of the project.

14. I do not have the geotechnical report dated June27, 2012. Where are the (2) borings located that were taken by Hynes and Assoc.?

Response: Geotechnical information was provided in Addendum No. 1 from pages 9-11 and as a separate document. The Geotechnical report prepared by Geo-Technology Associates, Inc. date June 2016 has been included to Addendum 2.

15. I see a note in addendum No.1, referencing unused soil from the project being disposed of to be included in the base bid. Does that include the soil being removed from Alternate No.1?

Response: See Sheet C-1 Note 16 for the disposition of unused soil on the project. Unit Cost No. 8 includes the disposal of unused soil from the project site at the direction of the Owner. The quantity shown in the Unit Cost includes excavation from the project in its entirety. The Owner may request as little as none or as much as all unused material be disposed from the project site.

16. It is unclear whether or not we are to take our price for all of the work east of Benge Rd and then add Allowances 1, 2 & 3 to that number and enter it on the first page of the bid form or whether DNREC will take our Base Bid price (from page 1 of bid form) and then add in the Allowances. This needs to be more clear so that all bidders on the project do the same thing with the allowance items. It would be easier for us if you had us price all work that wasn't included in allowances and submit that as a base bid and then let you add \$590,000 to everyone's bid. There would be a similar issue with Alternate 1 and Allowance #4, we need to know more clearly whether you want the \$150,000 added to our price for Alternate #1. There is potential for a \$740,000 mistake or variance between bids if someone misinterprets how these allowances are handled.

Response: The Bid Form has been revised to address this question. See the enclosed Bid Form for your use

17. Section 132950 page 5 3.3 B mentions "Top Down" method of construction, this method is typically required in a wet or environmentally sensitive area. The area where the timber bridge is to be constructed is in the same area designated as Staging Area for Workin' Bridges and will be completely disturbed and will likely have some type of stone or gravel base installed for stability of cranes, equipment, etc. To make the Timber Bridge contractor adhere to "Top Down" construction after all this disturbance takes place would be expensive and time consuming as compared to being able to start at the bridge abutment and drive all of the timber piles working your way back out towards the paved trail. Please consider eliminating this requirement from the specifications.

Response: Since the clearing and grubbing for the Staging Area for Workin' Bridges and the area below the boardwalk is going to occur, then the "Top Down" construction is not required for this project.

18. Plan Sheet C-11 Note 2 talks about intake and discharge pipes in the settlement tank being plugged, do you know how many of each there are and the size of intakes and discharges?

Response: It is assumed that 1-6" intake and 1-6" discharge pipe exist in the tank, and shall be included in the base bid. If additional pipes are encountered they shall be removed and plugged, as directed by the Owner. Cost for this additional work shall documented and be paid under a Change Order.

19. Is it safe to calculate the overall depth of the Settlement Tank from the Fam Lane Profile Plan Sheet C-6 or is the depth unknown?

Response: The contractor is advised not to scale the depth of the tank from the profile. As a follow up to the pre-bid meeting the Owner measured the depth of the tank to the top of the existing sediment to be approximately eleven (11) feet.

20. The 8' Shared Use Path next to the Bicycle Pad (west of Benge Rd) is shaded the same as the Scored PCC Pavement near the Settlement Tank please confirm these are the same type of pavement and please provide the Pavement Section (i.e. 8" Concrete over 6" GABC)

Response: The Shared Use Path next the Bicycle Pad shall be constructed of 6" of DelDOT Class B concrete over 4" of Graded Aggregate Base Course. A note has been added to Sheet C-9, and is included in this Addendum.

21. Plan Sheet ES-1 "Access to Paper Mill Bridge East Abutment" mentions restoration/redressing/addition of graded aggregate along the existing trail. This trail is not graded aggregate but hot mix asphalt, are we to include milling and overlaying this trail at the end of the project to restore to current conditions?

Response: The Department has added Allowance No. 5: Trail Repairs, totaling \$20,000 to the base bid. This allowance will be used as directed by the Owner for repairs to the HI Parcel Trail after it has been utilized as an access road. The Allowance specification and the bid forms have been updated and are included in this Addendum.

22. Just to clarify, we are to include the allowances monetary figures as shown on Page Four of the Bid Form (\$740,000) within our Lump Sum Price on Page One of the Bid Form. Also, is the Subcontractor Listing remaining the same? Your answer to one question asked during the Pre-Bid Meeting indicated that the number of listings might be reduced.

Response: See the response to Question #16 for your use.

23. Is the Temporary Road Way to be removed after construction is complete?

The contractor shall remove the Access Road at the completion of construction. The area shall be backfilled with excavated material, seeded and mulched. Sheet ES-1 has been revised to add this note, and is included in this Addendum.

24. Are there clearing of trees when installing the temporary roadway? If so, please specify.

The limits of clearing and grubbing are to the Limit of Disturbance.

25. Are excess soils generated from the road and parking lot construction to be exported off site or can they be stockpiled onsite? If soils can remain, where are they to be stockpiled?

This is considered the contractor's means and methods and should be considered when preparing his/her bid. Also, see the response to Question #15 for your use.

DRAWINGS:

- 1. **Drawing C-4:** Changed Shared Use Path to 6" PCC concrete.
- 2. **Drawing C-9:** Changed Shared Use Path to 6" PCC concrete and added notes clarifying the paving section for Farm Lane and the adjacent parking area.
- 3. **Drawing C-11:** Revised Note Reference. Revised Note 2, to clarify anticipated size of pipes removal and plugging to be included in the bid.
- 4. **Drawing C-16:** Changed Shared Use Path to 6" PCC concrete.
- 5. **Drawing ES-1:** Added Clear and Grubbing Note. Added Temporary Access Road Note, to clarify the disposition of the access road upon completion of construction. Revised Access To Paper Mill East Abutment, to include Allowance No. 5, for repairs.
- 6. **Drawing ES-2:** Revised Limit of Disturbance to include Bridge Staging Area and HI Trail.
- 7. **Drawings P-5 and P-6:** These are new sheets and identify the locations of the rail types indicated on Sheet P-4.

SPECIFICATIONS:

- 1. **Specification Section 012100, Allowances:** Allowance No. 5 Repairs to HI Trail, was added to the specification.
- 2. **Specification Section 012200, Unit Prices:** The quantity for Unit Price No. 4 Delaware No. 3 Stone was increased to 1,250 CY.
- 3. **Specification No. 004113, Bid Form:** The Bid Form was revised to include Allowance No. 5. And the Sub-Contractor List was revised.
- 4. **Specification T-600500, Design Build For Selected Project Elements:** The specification was revised to eliminate to the Contractor listing the Contractor's Engineer at the time of the bid, since the work covered under this specification is to be performed under and allowance.
- 5. **Specification 055213, Pipe and Tube Railings:** Paragraph 1.6D has been added to include information on the railing mock-up prior to final approval of the design and workmanship.

ADDITIONAL INFORMATION:

1. The Geotechnical Report Prepared by Geo-Technology Associates, Inc., dated June 10, 2016.

BID FORM

For Bids Due:	August 17, 2017 @ 1:00 PM	То:	Dept. of Natural Resources and Environmental Control Division of Parks and Recreation Office of Design and Development 89 Kings Highway, Dover DE 19901
Name of Bidde	er:		
	iness License No.: der's Delaware Business Lice		
(Other License	e Nos.):		
Phone Number	r: ()	_	Fax Number: ()
accordance then the Work is to l Bidding Docum equipment, sup	rewith, that he has visited the same performed, and that his bid in the nents without exception, hereby plies, transport and other facility	ite and hat s based up propose ties require	erstands the Bidding Documents and that this bid is made in as familiarized himself with the local conditions under which pon the materials, systems and equipment described in the s and agrees to provide all labor, materials, plant, red to execute the work described by the aforesaid e Bid shall include Allowances 1, 2, 3, and 5:
\$			
	(\$)

BID FORM

ALTERNATES

Alternate prices conform to applicable project specification section. Refer to specifications for a complete description of the following Alternates. An "ADD" or "DEDUCT" amount is indicated by the crossed out part that does not apply.

ALTERNATE No. 1:	Proposed construction to the west of Benge Road, including the Farm Lane parking lot, Museum Drive, and Museum Drive parking lot. (Alternate 1 shall include Allowance 4.)			
Add/Deduct:	\$			
	(\$)		
No. of Days to Complete Alternate 1:				

BID FORM

UNIT PRICES

Unit prices conform to applicable project specification section. Refer to the specifications for a complete description of the following Unit Prices:

UNIT PRICE No. 1:	Undercut Excavation	Add or Deduct	\$ _ /cy
UNIT PRICE No. 2	DelDOT Borrow Type B	Add or Deduct	\$ _ /cy
UNIT PRICE No. 3	DelDOT Borrow Type A	Add or Deduct	\$ _ /cy
UNIT PRICE No. 4	Delaware No. 3 Stone	Add or Deduct	\$ _ /cy
UNIT PRICE No. 5	Geotextile Separation Fabric	Add or Deduct	\$ _ /sy
UNIT PRICE No. 6	Rock Excavation (non-Structural)	Add or Deduct	\$ _ /cy
UNIT PRICE No. 7	Silt Fence	Add or Deduct	\$ _ /lf
UNIT PRICE No. 8	Offsite Soil Disposal	Add or Deduct	\$ _ /cy
UNIT PRICE No. 9	Flowable Fill	Add or Deduct	\$ _/cy

BID FORM

ALLOWANCES

The following allowances are included in the Base Bid. Allowances conform to applicable project specification sections. Refer to the specifications for a complete description of the following Allowances:

Allowance No. 1: Path connection to the existing office. (Base Bid)

Two Hundred Thousand Dollars and Zero Cents (\$ 200,000 Allowance No. 2: Construction of the Mill Race deck-over structure as described in Sections 600500 and 600600. (Base Bid) Three Hundred Fifty Thousand Dollars and Zero Cents 350,000 (\$ Allowance No. 3: Mill Race inspection and repair. (Base Bid) Forty Thousand Dollars and Zero Cents (\$ 40,000 Allowance No. 4: Removal and replacement of existing fence. (This allowance is warranted if the owner selects Alternate 1.) \$One-Hundred and Fifty Thousand Dollars and Zero Cents 150,000 Allowance No. 5: HI Parcel Trail repair. (Base Bid) Twenty Thousand Dollars and Zero Cents 20,000 (\$

BID FORM

I/We acknowledge Addendums numbered and the price(s) submitted include any cost/schedule impact they may have.				
This bid shall remain valid and cannot be withdrawn for thirty (30) days shall abide by the Bid Security forfeiture provisions. Bid Security is atta				
The Owner shall have the right to reject any or all bids, and to waive any	y informality o	or irregularity in any bid received.		
This bid is based upon work being accomplished by the Sub-Contractors	s named on the	e list attached to this bid.		
Should I/We be awarded this contract, I/We pledge to achieve substantial days of the Notice to Proceed.	al completion	of all the work withincalendar		
The undersigned represents and warrants that he has complied and shall comply with all requirements of local, state, and national laws; that no legal requirement has been or shall be violated in making or accepting this bid, in awarding the contract to him or in the prosecution of the work required; that the bid is legal and firm; that he has not, directly or indirectly, entered into any agreement, participated in any collusion, or otherwise taken action in restraint of free competitive bidding.				
Upon receipt of written notice of the acceptance of this Bid, the Bidder's agreement in the required form and deliver the Contract Bonds, and Insu Documents.				
I am / We are an Individual / a Partnership / a Corporation				
By Trading as				
(Individual's/General Partner's /Corporate Name)				
(State of Corporation)				
Business Address:				
Witness:	By:			
WithCSS.	Бу.	(Authorized Signature)		
(Seal)				
<u>ATACHMENTS</u>		(Title) Date:		
Sub-Contractor List				
Non-Collusion Statement				
Bid Security				
(Others as Required by Project Manual)				

BID FORM

SUBCONTRACTOR LIST

In accordance with Title 29, Chapter 6962 (d)(10)b <u>Delaware Code</u>, the following sub-contractor listing must accompany the bid submittal. The name and address of the sub-contractor **must be listed for each category** where the bidder intends to use a sub-contractor to perform that category of work. In order to provide full disclosure and acceptance of the bid by the *Owner*, it is required that bidders list themselves as being the sub-contractor for all categories where he/she is qualified and intends to perform such work.

	Subcontractor Category	Subcontractor	Address (City & State)	Subcontractors tax payer ID # or Delaware Business license #
1.	Traffic Control			
2.	Paving			
3.	Structural Concrete			
4.	Non-Structural			
	Concrete			
5.	Railing			
6.	Site Inspection			
7.	Timber Boardwalk			
	Contractor			
8.	Mill Race Inspection			
	-			

BID FORM

NON-COLLUSION STATEMENT

This is to certify that the undersigned bidder has neither directly nor indirectly, entered into any agreement, participated in any collusion or otherwise taken any action in restraint of free competitive bidding in connection with this proposal submitted this date to the Office of Design and Development, Division of Parks and Recreation.

All the terms and conditions of the Auburn Heights Preserve –Paper Mill Bridge project have been thoroughly examined and are understood.

NAME OF BIDDER			
AUTHORIZED REPRESENTATIVE (TYPED):			
AUTHORIZED REPRESENTATIVE (SIGNATURE):			
TITLE:			
ADDRESS OF BIDDER:			
E-MAIL:			
PHONE NUMBER:			
Sworn to and Subscribed before me this	day of	of 20	
My commission expires	. NOTARY	PULIC	

THIS PAGE MUST BE SIGNED AND NOTORIZED FOR YOUR BID TO BE CONSIDERED.

AFFIDAVIT OF EMPLOYEE DRUG TESTING PROGRAM

4104 Regulations for the Drug Testing of Contractor and Subcontractor Employees Working on Large Public Works Projects requires that Contractors and Subcontractors implement a program of mandatory drug testing for Employees who work on Large Public Works Contracts funded all or in part with public funds.

We hereby certify that we have in place or will implement during the entire term of the contract a Mandatory Drug Testing Program for our employees on the jobsite that complies with this regulation:

Contractor/Subcontractor Name:		
Contractor/Subcontractor Address:		
Authorized Representative (typed or printed):		
Authorized Representative (signature):		
Title:		
Sworn to and Subscribed before me this	day of	20
My Commission expires	NOTARY PUBLIC	

THIS PAGE MUST BE SIGNED AND NOTARIZED FOR YOUR BID TO BE CONSIDERED.

SECTION 012100 - ALLOWANCES

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and other Division 01 Specification Sections, apply to this Section.

1.2 SUMMARY

- A. Section includes administrative and procedural requirements governing allowances.
- B. Selected materials and equipment are specified in the Contract Documents by Allowances. In some cases, these Allowances include installation. Allowances have been established in lieu of additional requirements and to defer selection of actual materials and equipment to a later date when additional information is available for evaluation. If necessary, additional requirements will be issued by Proposal Request.
- C. Types of allowances include the following:
 - 1. Lump-sum allowances.
 - 2. Unit-cost Allowances.
 - 3. Contingency Allowances.

D. Related Requirements:

1. Section 012200 "Unit Prices" for procedures for using unit prices.

1.3 SELECTION AND PURCHASE

- A. At the earliest practical date after award of the Contract, advise Owner of the date when final selection and purchase of each product or system described by an allowance must be completed to avoid delaying the Work.
- B. At Owner's request, obtain proposals for each allowance for use in making final selections. Include recommendations that are relevant to performing the Work.
- C. Purchase products and systems selected in writing by Owner from the designated supplier.

1.4 ACTION SUBMITTALS

A. Submit proposals for purchase of products or systems included in allowances, in the form specified for Change Orders.

ALLOWANCES 012100 - 1

1.5 INFORMATIONAL SUBMITTALS

- A. Submit invoices or delivery slips to show actual quantities of materials delivered to the site for use in fulfillment of each allowance.
- B. Submit time sheets and other documentation to show labor time and cost for installation of allowance items that include installation as part of the allowance.
- C. Coordinate and process submittals for allowance items in same manner as for other portions of the Work.

1.6 COORDINATION

A. Coordinate allowance items with other portions of the Work. Furnish templates as required to coordinate installation.

1.7 LUMP-SUM ALLOWANCES

- A. Allowance shall include cost to Contractor of specific products and materials ordered by Owner or selected by Architect under allowance and shall include taxes, freight, and delivery to Project site.
- B. Unless otherwise indicated, Contractor's costs for receiving and handling at Project site, labor, installation, overhead and profit, and similar costs related to products and materials selected by Architect under allowance shall be included as part of the Contract Sum and not part of the allowance.
- C. Unused Materials: Return unused materials purchased under an allowance to manufacturer or supplier for credit to Owner, after installation has been completed and accepted.
 - 1. If requested by Architect, retain and prepare unused material for storage by Owner. Deliver unused material to Owner's storage space as directed.

1.8 ADJUSTMENT OF ALLOWANCES

- A. Allowance Adjustment: To adjust allowance amounts, prepare a Change Order proposal based on the difference between purchase amount and the allowance, multiplied by final measurement of work-in-place where applicable. If applicable, include reasonable allowances for cutting losses, tolerances, mixing wastes, normal product imperfections, and similar margins.
 - 1. Include installation costs in purchase amount only where indicated as part of the allowance.
 - 2. If requested, prepare explanation and documentation to substantiate distribution of overhead costs and other margins claimed.

012100 - 2 ALLOWANCES

- 3. Submit substantiation of a change in scope of work, if any, claimed in Change Orders related to unit-cost allowances.
- 4. Owner reserves the right to establish the quantity of work-in-place by independent quantity survey, measure, or count.
- B. Submit claims for increased costs because of a change in scope or nature of the allowance described in the Contract Documents, whether for the purchase order amount or Contractor's handling, labor, installation, overhead, and profit.
 - 1. Do not include Contractor's or subcontractor's indirect expense in the Change Order cost amount unless it is clearly shown that the nature or extent of work has changed from what could have been foreseen from information in the Contract Documents.
 - 2. No change to Contractor's indirect expense is permitted for selection of higher- or lower-priced materials or systems of the same scope and nature as originally indicated.

1.9 Contingency Allowances:

- A. Use the Contingency Allowance only as directed for the Owner's purposes and only by Change Orders that indicate amounts to be charged to the Allowance.
- B. Change Orders authorizing use of funds from the Contingency Allowance, for purchase of products and equipment, will include Contractor's related costs and reasonable overhead and profit margins. These related costs include delivery, installation, taxes, insurance, equipment rental, and similar costs.
- C. At Project Closeout, credit unused amounts remaining in the Contingency Allowance to the Owner by Change Order.

PART 2 - PRODUCTS (Not Used)

PART 3 - EXECUTION

3.1 EXAMINATION

A. Examine products covered by an allowance promptly on delivery for damage or defects. Return damaged or defective products to manufacturer for replacement.

3.2 PREPARATION

A. Coordinate materials and their installation for each allowance with related materials and installations to ensure that each allowance item is completely integrated and interfaced with related work.

ALLOWANCES 012100 - 3

3.3 SCHEDULE OF ALLOWANCES

- A. Allowance No. 1 Lump-Sum Allowance: Include, the sum of \$200,000 for the path connection to the existing office.
 - 1. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit and engineering.
- B. Allowance No. 2 Lump Sum Allowance: Include, the sum of \$350,000 for the construction of the Mill Race deck-over structure as described in Sections 600500 and 600600.
 - 1. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit and engineering.
- C. Allowance No. 3 Lump Sum Allowance: Include, the sum of \$40,000.00 for inspection of and repairs to the mill race below the deck-over structure according to the Owner's direction.
 - 1. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit.
- D. Allowance No. 4 Lump Sum Allowance to Add/Alternate No. 1: Include, the sum of \$150,000.00 for the removal and replacement of existing fence and gates.
 - 1. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit.
- E. Allowance No. 5 Lump Sum Allowance: Include, the sum of \$20,000 for repairs to the HI Parcel Trail according to the Owner's direction.
 - 1. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit.

END OF SECTION 012100

012100 - 4 ALLOWANCES

SECTION 012200 - UNIT PRICES

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and other Division 1 Specification Sections, apply to this Section.

1.2 SUMMARY

- A. This Section includes administrative and procedural requirements for unit prices.
- B. Related Sections include the following:
 - 1. Division 1 Section "Contract Modification Procedures" for procedures for submitting and handling Change Orders.

1.3 DEFINITIONS

A. Unit price is an amount proposed by bidders, stated on the Bid Form, as a price per unit of measurement for materials or services added to or deducted from the Contract Sum by appropriate modification, if estimated quantities of Work required by the Contract Documents are increased or decreased.

1.4 PROCEDURES

- A. Unit prices include all necessary material, plus cost for delivery, installation, insurance, applicable taxes, overhead, and profit.
- B. Measurement and Payment: Refer to individual Specification Sections for work that requires establishment of unit prices. Methods of measurement and payment for unit prices are specified in those Sections.
- C. Owner reserves the right to reject Contractor's measurement of work-in-place that involves use of established unit prices and to have this work measured, at Owner's expense, by an independent surveyor acceptable to Contractor.
- D. List of Unit Prices: A list of unit prices is included at the end of this Section. Specification Sections referenced in the schedule contain requirements for materials described under each unit price.

PART 2 - PRODUCTS (Not Used)

UNIT PRICES 012200 - 1

PART 3 - EXECUTION

3.1	LIST OF UNIT PRICES
A.	 Unit Price No. 1 – Undercut Excavation Description: Excavation of poor subgrade materials as determined by the owner. Unit of Measurement: Cubic Yard (CY) Include 200 CY x Unit Price \$/CY = \$
В.	 Unit Price No. 2 – DelDOT Borrow Type B Description: Backfill material for undercut in wet areas. Unit of Measurement: Cubic Yard (CY) Include 200 CY x Unit Price \$/CY = \$
C.	 Unit Price No. 3 – DelDOT Borrow Type A Description: Backfill material for undercut in all areas other than wet areas. Unit of Measurement: Cubic Yard (CY) Include 200 CY x Unit Price \$/CY = \$
D.	 Unit Price No. 4 – Delaware No. 3 Stone Description: Backfill material for bridging undercut areas. Unit of Measurement: Cubic Yard (CY) Include 1,250 CY x Unit Price \$/CY = \$
E.	 Unit Price No. 5 – Geotextile Separation Fabric Description: Separation Fabric to be placed under borrow in undercut areas Unit of Measurement: Square Yard (SY) Include 500 SY x Unit Price \$/SY = \$
F.	 Unit Price No. 6 – Rock Excavation (Non-Structural) Description: Excavation of rock material in areas other than the proposed structures. Unit of Measurement: Cubic Yard (CY) Include 200 CY x Unit Price \$/CY = \$
G.	 Unit Price No. 7 – Silt Fence Description: Furnishing, installing and maintenance of silt fence at locations other than those shown on the plans at the direction of the Owner. Unit of Measurement: Linear Feet (LF) Include 500 LF x Unit Price \$/LF = \$
Н.	 Unit Price No. 8 – Offsite Soil Disposal Description: Disposal of unused soil from the project site at the direction of the Owner. Unit of Measurement: Cubic Yard (CY) Include 2400 CY x Unit Price \$ /CY = \$

END OF SECTION 012200

012200 - 2 UNIT PRICES

SECTION 055213 - PIPE AND TUBE RAILINGS

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and Division 01 Specification Sections, apply to this Section.

1.2 SUMMARY

- A. Section Includes:
 - 1. Steel pipe railings.
- B. Related Sections:
 - 1. Section 099113 "Exterior Painting" for painting of steel tube railings.

1.3 PERFORMANCE REQUIREMENTS

- A. Delegated Design: Design railings, including comprehensive engineering analysis by a qualified professional engineer, using performance requirements and design criteria indicated.
- B. General: In engineering railings to withstand structural loads indicated, determine allowable design working stresses of railing materials based on the following:
 - 1. Steel: 72 percent of minimum yield strength.
 - 2. Stainless Steel: 60 percent of minimum yield strength.
- C. Structural Performance: Railings shall withstand the effects of gravity loads and the following loads and stresses within limits and under conditions indicated:
 - 1. Handrails and Top Rails of Guards:
 - a. Uniform load of 50 lbf/ ft. (0.73 kN/m) applied in any direction.
 - b. Concentrated load of 200 lbf (0.89 kN) applied in any direction.
 - c. Uniform and concentrated loads need not be assumed to act concurrently.
 - 2. Infill of Guards:
 - a. Concentrated load of 50 lbf (0.22 kN) applied horizontally on an area of 1 sq. ft. (0.093 sq. m).
 - b. Infill load and other loads need not be assumed to act concurrently.

- D. Thermal Movements: Allow for thermal movements from ambient and surface temperature changes acting on exterior metal fabrications by preventing buckling, opening of joints, overstressing of components, failure of connections, and other detrimental effects.
 - 1. Temperature Change: 120 deg F (67 deg C), ambient; 180 deg F (100 deg C), material surfaces.
- E. Control of Corrosion: Prevent galvanic action and other forms of corrosion by insulating metals and other materials from direct contact with incompatible materials.

1.4 ACTION SUBMITTALS

- A. Product Data: For the following:
 - 1. Railing brackets.
 - 2. Grout, anchoring cement, and paint products.
- B. Shop Drawings: Include plans, elevations, sections, details, and attachments to other work.
- C. Delegated-Design Submittal: For installed products indicated to comply with performance requirements and design criteria, including analysis data signed and sealed by the qualified professional engineer responsible for their preparation.

1.5 INFORMATIONAL SUBMITTALS

- A. Qualification Data: For qualified professional engineer.
- B. Mill Certificates: Signed by manufacturers of stainless-steel products certifying that products furnished comply with requirements.
- C. Welding certificates.
- D. Paint Compatibility Certificates: From manufacturers of topcoats applied over shop primers certifying that shop primers are compatible with topcoats.
- E. Product Test Reports: Based on evaluation of comprehensive tests performed by a qualified testing agency, according to ASTM E 894 and ASTM E 935.

1.6 QUALITY ASSURANCE

- A. Source Limitations: Obtain each type of railing from single source from single manufacturer.
- B. Welding Qualifications: Qualify procedures and personnel according to AWS D1.1/D1.1M, "Structural Welding Code Steel."
- C. Welding Qualifications: Qualify procedures and personnel according to the following:
 - 1. AWS D1.1/D1.1M, "Structural Welding Code Steel."

- D. Mockups: Build mockups to demonstrate aesthetic effects and to set quality standards for materials and execution.
 - 1. Build mockup of typical railing section 4-feet high by 24-feet long as shown on Drawings.
 - 2. Protect accepted mockups with polyethylene sheet membrane.
 - 3. Approval of mockups does not constitute approval of deviations from the Contract Documents contained in mockups unless Architect specifically approves such deviations in writing.
 - 4. Subject to compliance with requirements, approved mockups may become part of the completed Work if undisturbed at time of Substantial Completion.

1.7 PROJECT CONDITIONS

A. Field Measurements: Verify actual locations of walls and other construction contiguous with metal fabrications by field measurements before fabrication.

1.8 COORDINATION AND SCHEDULING

- A. Coordinate selection of shop primers with topcoats to be applied over them. Comply with paint and coating manufacturers' written recommendations to ensure that shop primers and topcoats are compatible with one another.
- B. Coordinate installation of anchorages for railings. Furnish setting drawings, templates, and directions for installing anchorages, including sleeves, concrete inserts, anchor bolts, and items with integral anchors, that are to be embedded in concrete or masonry. Deliver such items to Project site in time for installation.
- C. Schedule installation so wall attachments are made only to completed walls. Do not support railings temporarily by any means that do not satisfy structural performance requirements.

PART 2 - PRODUCTS

2.1 MANUFACTURERS

2.2 METALS, GENERAL

- A. Metal Surfaces, General: Provide materials with smooth surfaces, without seam marks, roller marks, rolled trade names, stains, discolorations, or blemishes.
- B. Brackets, Flanges, and Anchors: Cast or formed metal of same type of material and finish as supported rails unless otherwise indicated.

2.3 STEEL AND IRON

- A. Pipe: ASTM A 53/A 53M, Type F or Type S, Grade A, Standard Weight (Schedule 40), unless another grade and weight are required by structural loads.
 - 1. Provide galvanized finish for exterior installations and where indicated.
- B. Plates, Shapes, and Bars: ASTM A 36/A 36M.
- C. Cast Iron: Either gray iron, ASTM A 48/A 48M, or malleable iron, ASTM A 47/A 47M, unless otherwise indicated.

2.4 FASTENERS

- A. General: Provide the following:
 - 1. Hot-Dip Galvanized Railings: Type 304 stainless-steel or hot-dip zinc-coated steel fasteners complying with ASTM A 153/A 153M or ASTM F 2329 for zinc coating.
- B. Fasteners for Anchoring Railings to Other Construction: Select fasteners of type, grade, and class required to produce connections suitable for anchoring railings to other types of construction indicated and capable of withstanding design loads.
- C. Fasteners for Interconnecting Railing Components:
 - 1. Provide concealed fasteners for interconnecting railing components and for attaching them to other work, unless otherwise indicated.
 - 2. Provide Phillips flat-head machine screws for exposed fasteners unless otherwise indicated.

2.5 MISCELLANEOUS MATERIALS

- A. Welding Rods and Bare Electrodes: Select according to AWS specifications for metal alloy welded.
- B. Etching Cleaner for Galvanized Metal: Complying with MPI#25.
- C. Galvanizing Repair Paint: High-zinc-dust-content paint complying with SSPC-Paint 20 and compatible with paints specified to be used over it.
- D. Shop Primers: Provide primers that comply with Section 099113 "Exterior Painting".
- E. Intermediate Coats and Topcoats: Provide products that comply with Section 099113 "Exterior Painting".
- F. Bituminous Paint: Cold-applied asphalt emulsion complying with ASTM D 1187.
- G. Nonshrink, Nonmetallic Grout: Factory-packaged, nonstaining, noncorrosive, nongaseous grout complying with ASTM C 1107. Provide grout specifically recommended by manufacturer for interior and exterior applications.

2.6 FABRICATION

- A. General: Fabricate railings to comply with requirements indicated for design, dimensions, member sizes and spacing, details, finish, and anchorage, but not less than that required to support structural loads.
- B. Assemble railings in the shop to greatest extent possible to minimize field splicing and assembly. Disassemble units only as necessary for shipping and handling limitations. Clearly mark units for reassembly and coordinated installation. Use connections that maintain structural value of joined pieces.
- C. Cut, drill, and punch metals cleanly and accurately. Remove burrs and ease edges to a radius of approximately 1/32 inch (1 mm) unless otherwise indicated. Remove sharp or rough areas on exposed surfaces.
- D. Form work true to line and level with accurate angles and surfaces.
- E. Fabricate connections that will be exposed to weather in a manner to exclude water. Provide weep holes where water may accumulate.
- F. Cut, reinforce, drill, and tap as indicated to receive finish hardware, screws, and similar items.
- G. Connections: Fabricate railings with welded connections unless otherwise indicated.
- H. Welded Connections: Cope components at connections to provide close fit, or use fittings designed for this purpose. Weld all around at connections, including at fittings.
 - 1. Use materials and methods that minimize distortion and develop strength and corrosion resistance of base metals.
 - 2. Obtain fusion without undercut or overlap.
 - 3. Remove flux immediately.
 - 4. At exposed connections, finish exposed surfaces smooth and blended so no roughness shows after finishing and welded surface matches contours of adjoining surfaces.
- I. Form changes in direction as follows:
 - 1. By radius bends of radius indicated
- J. Bend members in jigs to produce uniform curvature for each configuration required; maintain cross section of member throughout entire bend without buckling, twisting, cracking, or otherwise deforming exposed surfaces of components.
- K. Close exposed ends of railing members with prefabricated end fittings.
- L. Provide wall returns at ends of rail-mounted handrails unless otherwise indicated. Close ends of returns unless clearance between end of rail and wall is 1/4 inch (6 mm) or less.
- M. Brackets, Flanges, Fittings, and Anchors: Provide wall brackets, flanges, miscellaneous fittings, and anchors to interconnect railing members to other work unless otherwise indicated.

- 1. At brackets and fittings fastened to plaster or gypsum board partitions, provide crushresistant fillers, or other means to transfer loads through wall finishes to structural supports and prevent bracket or fitting rotation and crushing of substrate.
- N. Provide inserts and other anchorage devices for connecting railings to concrete or masonry work. Fabricate anchorage devices capable of withstanding loads imposed by railings. Coordinate anchorage devices with supporting structure.

2.7 FINISHES, GENERAL

- A. Comply with NAAMM's "Metal Finishes Manual for Architectural and Metal Products" for recommendations for applying and designating finishes.
- B. Protect mechanical finishes on exposed surfaces from damage by applying a strippable, temporary protective covering before shipping.

2.8 STEEL AND IRON FINISHES

A. Galvanized Railings:

- 1. Hot-dip galvanize exterior steel and iron railings, including hardware, after fabrication.
- 2. Comply with ASTM A 123/A 123M for hot-dip galvanized railings.
- 3. Comply with ASTM A 153/A 153M for hot-dip galvanized hardware.
- 4. Do not quench or apply post galvanizing treatments that might interfere with paint adhesion.
- 5. Fill vent and drain holes that will be exposed in the finished Work, unless indicated to remain as weep holes, by plugging with zinc solder and filing off smooth.
- B. For galvanized railings, provide hot-dip galvanized fittings, brackets, fasteners, sleeves, and other ferrous components.
- C. Preparing Galvanized Railings for Shop Priming: After galvanizing, thoroughly clean railings of grease, dirt, oil, flux, and other foreign matter, and treat with etching cleaner.
- D. Preparation for Shop Priming: Prepare uncoated ferrous-metal surfaces to comply with SSPC-SP 6/NACE No. 3, "Commercial Blast Cleaning.
 - 1. Shop prime uncoated railings with universal shop primer unless indicated.

PART 3 - EXECUTION

3.1 EXAMINATION

A. Examine bridges and concrete decking, which are to receive anchors, to verify that installations are plumb, meet ramping requirements and are ready for railing installation.

3.2 INSTALLATION, GENERAL

- A. Fit exposed connections together to form tight, hairline joints.
- B. Perform cutting, drilling, and fitting required for installing railings. Set railings accurately in location, alignment, and elevation; measured from established lines and levels and free of rack.
 - 1. Do not weld, cut, or abrade surfaces of railing components that have been coated or finished after fabrication and that are intended for field connection by mechanical or other means without further cutting or fitting.
 - 2. Set posts plumb within a tolerance of 1/16 inch in 3 feet (2 mm in 1 m).
 - 3. Align rails so variations from level for horizontal members and variations from parallel with rake of steps and ramps for sloping members do not exceed 1/4 inch in 12 feet (5 mm in 3 m).
- C. Adjust railings before anchoring to ensure matching alignment at abutting joints.
- D. Fastening to In-Place Construction: Use anchorage devices and fasteners where necessary for securing railings and for properly transferring loads to in-place construction.

3.3 RAILING CONNECTIONS

A. Welded Connections: Use fully welded joints for permanently connecting railing components. Comply with requirements for welded connections in "Fabrication" Article whether welding is performed in the shop or in the field.

3.4 ANCHORING POSTS

- A. Anchor posts to metal surfaces with oval flanges, angle type, or floor type as required by conditions, connected to posts and to metal supporting members as follows:
 - 1. For steel pipe railings, weld flanges to post and bolt to metal, wood or concrete supporting surfaces.
 - 2. Provide non-shrink grout where anchoring surfaces are not plumb. Place grout to comply with anchoring material manufacturer's written instructions.

3.5 ATTACHING RAILINGS

- A. Attach railings to posts with post brackets. Provide brackets with 2-1/4-inch (38-mm) clearance from inside face of handrail and finished rail surface. Locate brackets as indicated or, if not indicated, at spacing required to support structural loads.
 - 1. Use ½-inch diameter round galvanized bar bracket.
 - 2. Locate brackets as indicated or, if not indicated, at spacing required to support structural loads.
 - 3. Shop weld anchors as indicated on drawings. Grind welds smooth. Touch-up welds with primer.

3.6 ADJUSTING AND CLEANING

- A. Touchup Painting: Immediately after erection, clean field welds, bolted connections, and abraded areas of shop paint, and paint exposed areas with the same material as used for shop painting to comply with SSPC-PA 1 for touching up shop-painted surfaces.
 - 1. Apply by brush or spray to provide a minimum 2.0-mil (0.05-mm) dry film thickness.
- B. Touchup Painting: Cleaning and touchup painting of field welds, bolted connections, and abraded areas of shop paint are specified in Section 099113 "Exterior Painting" and Section 099123 "Interior Painting."
- C. Galvanized Surfaces: Clean field welds, bolted connections, and abraded areas and repair galvanizing to comply with ASTM A 780.

3.7 PROTECTION

A. Protect finishes of railings from damage during construction period with temporary protective coverings approved by railing manufacturer. Remove protective coverings at time of Substantial Completion.

END OF SECTION 055213

T600500 - DESIGN-BUILD FOR SELECTED PROJECT ELEMENTS

In the interest of reducing the cost of the project, the Contractor shall use a design build process for the following project elements (heretofore referred to as the Design Build Elements)

1. Design and Construction of the Deck Over Structure spanning over the existing mill race and connecting Benge Road to the proposed parking lot constructed on the existing settling tank. The design of the Deck Over structure shall include but is not limited to the design of the precast concrete planks, concrete substructure and the micro piles.

All project elements other than those specifically cited above as Design Build Element(s) shall be constructed in accordance with the provided plans and specifications.

The Contractor shall design and construct the Design Build Element(s) in accordance with the performance specifications, plans, details and design criteria outlined in the contract documents, and shall comply with the latest AASHTO specifications.

The Contractor shall be responsible for reviewing existing site conditions. The provided Geotechnical Reports are for the Contractor's information. The contractor shall be responsible for conducting additional subsurface investigation as the contractor deems necessary with respect to the Design Build Elements. No additional payment shall be made by to the Owner to the Contractor for additional subsurface investigations.

The design effort shall be led by an engineering firm which has successfully completed three projects of similar size and scope in the last five years. This firm is identified in this special provision as the Contractor's Engineer. The Contractor's Engineer shall be a registered Professional Engineer in the State of Delaware and shall be responsible for assembling, signing and sealing the design build package as a whole.

The Contractor shall identify and submit qualifications for the Contractor's Engineer in accordance with the requirements outlined in the special provisions. Upon award of Contract the contractor shall be required to submit documentation that the Contractor's Engineer is a registered professional engineer in the state of Delaware and evidence of successfully completing the three projects as stated above. Documentation shall include; name of project, Owner, Owner contact information and date of project.

All design performed by parties other than the Contractor's Engineer (including but not limited to micro piles or precast planks) shall be reviewed and approved by the Contractor's Engineer prior to being submitted for review and approval by the Owner and/or the Owner's representative. Such submittals shall include complete plans and calculations signed and sealed by a Professional Engineer registered in the State of Delaware. The Contractor's Engineer shall stamp each submission as approved prior to forwarding the submission to the Owner and/or the Owner's representative. The contractor's engineer shall assemble, sign and seal the submitted package.

Each element of the Design Build Elements submissions shall be sealed by a Professional Engineer licensed in the State of Delaware.

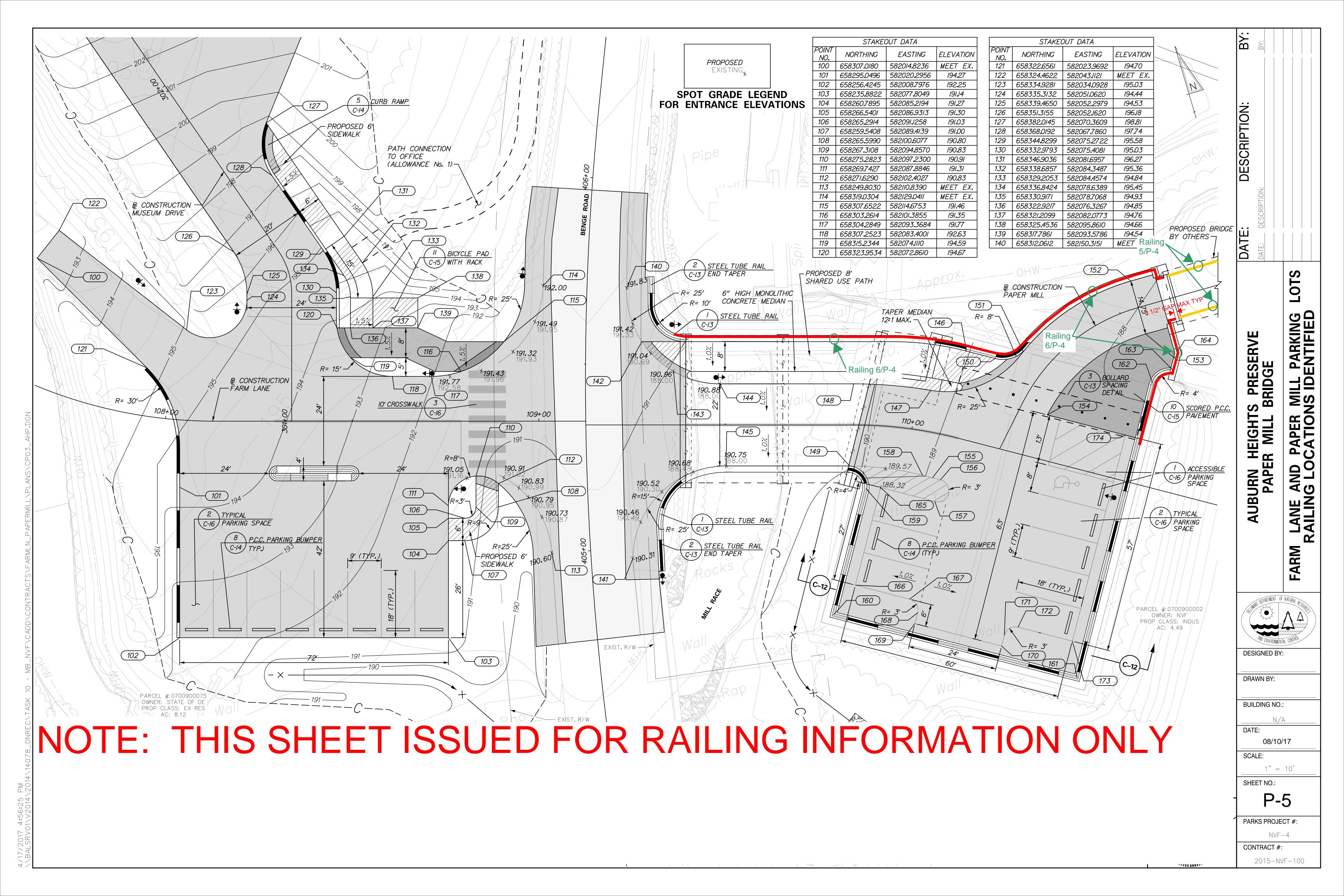
After Notice to Proceed, the selected Contractor shall submit plans, shop drawings, calculations, reports, working drawings, erection drawings, agency, and utility coordination drawings/procedures; product/material data; and other documentation for the Design Build Elements to the Owner and/or the Owner's representative for review and approval. The Owner and/or their representative will review the submission for compliance with the performance specifications and design requirements. The Owner and/or their representative shall have 10 calendar days to provide comments or approve the submission. If the Owner and/or their representative provides comments, the contractor shall address the comments in writing, modify the submission, and resubmit to the Owner and/or their representative. Multiple submissions and responses to comments may be necessary. After all comments are resolved, the Owner and/or their representative will approve the submission. The Contractor shall not begin construction of the Design Build Element(s) prior to the shop drawings being approved by the Owner or the Owner's representative. The contractor shall take into consideration the review time when scheduling the construction.

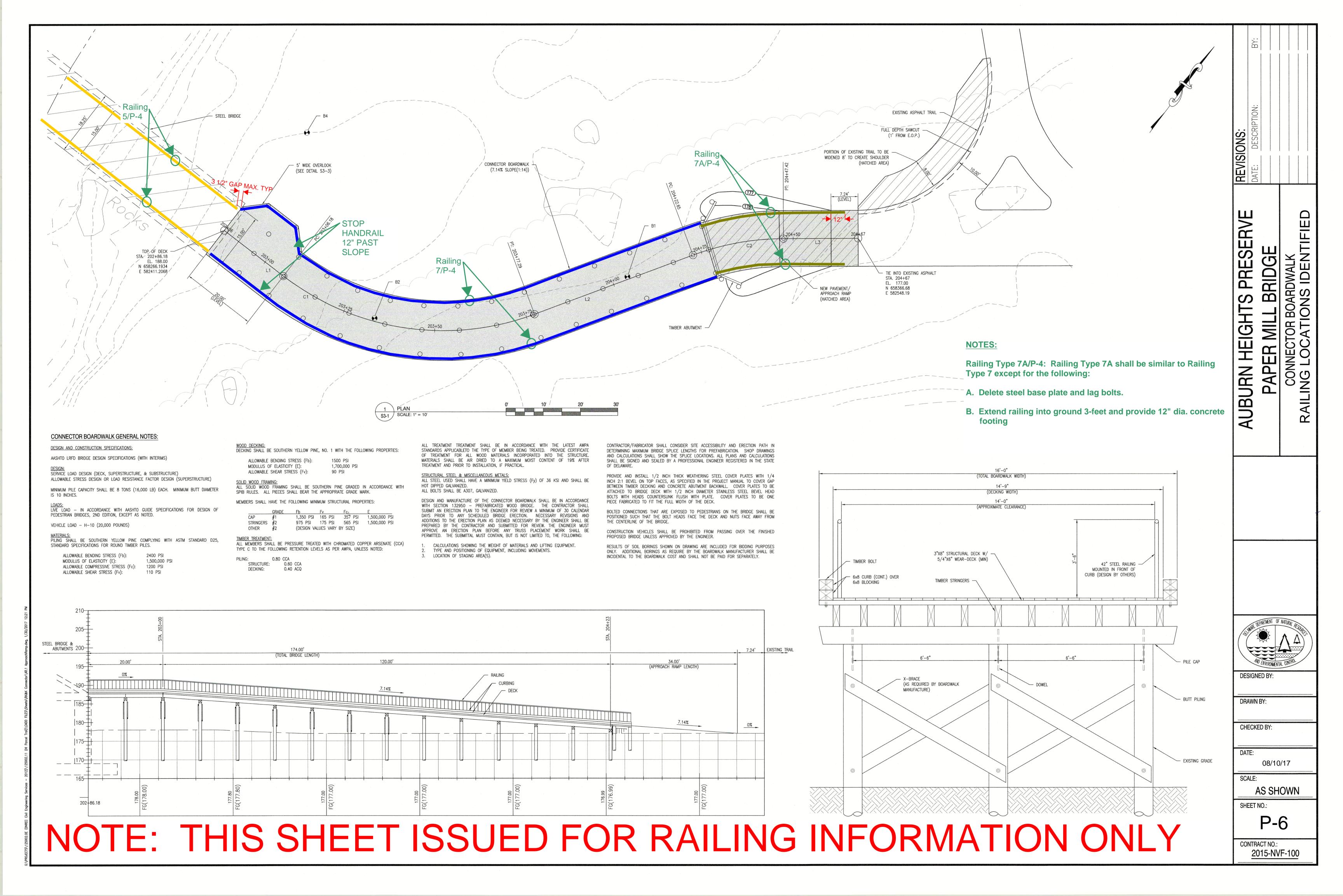
Approval of the submission indicates only that the design, to the extent reviewed by the Owner and/or their representative, meets the performance specifications and design requirements and does not relieve the contractor of responsibility and risk for the Design Build Element(s).

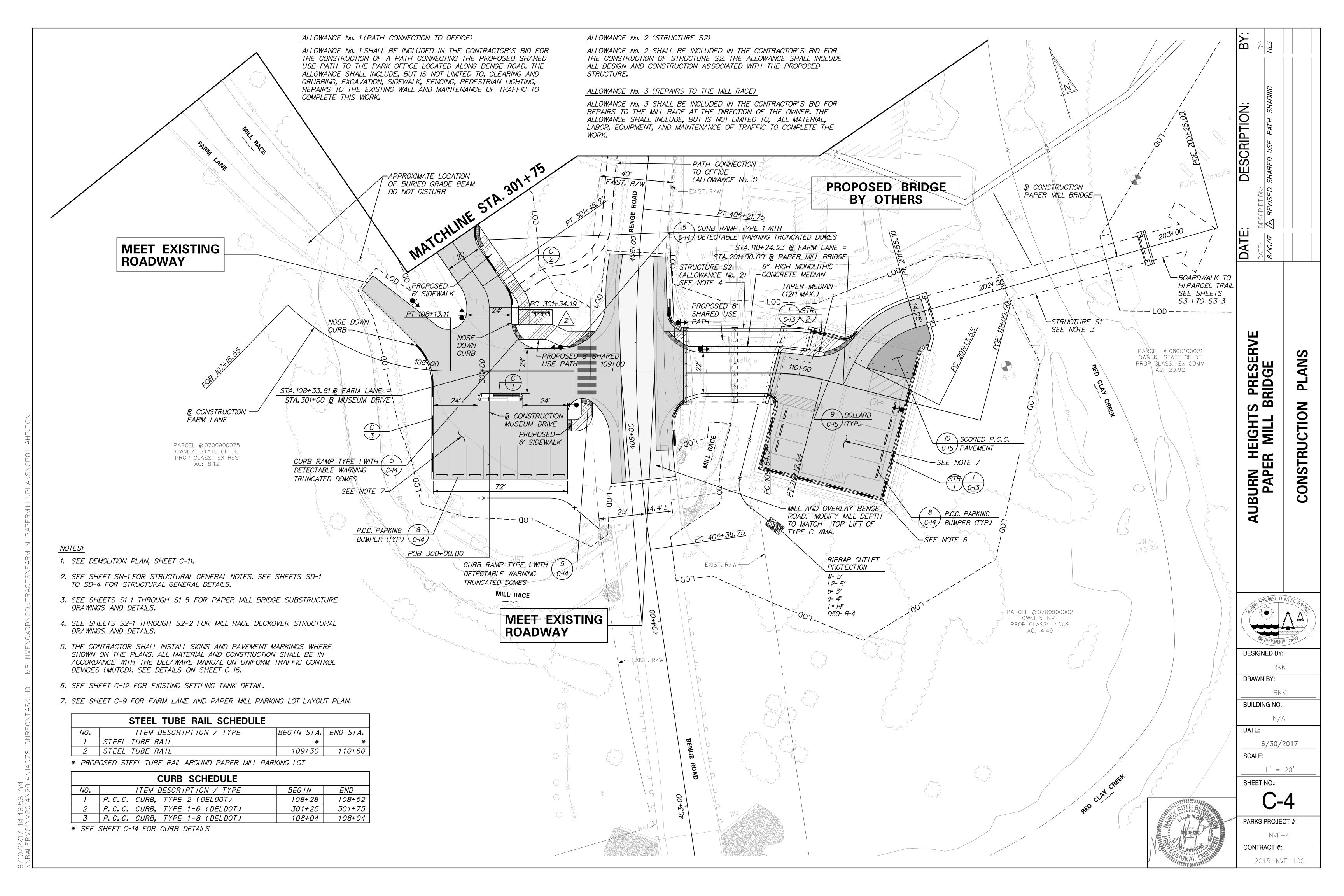
The Contractor shall be responsible for obtaining all permits and permit modifications necessary to complete the proposed work related to the Design Build Element(s).

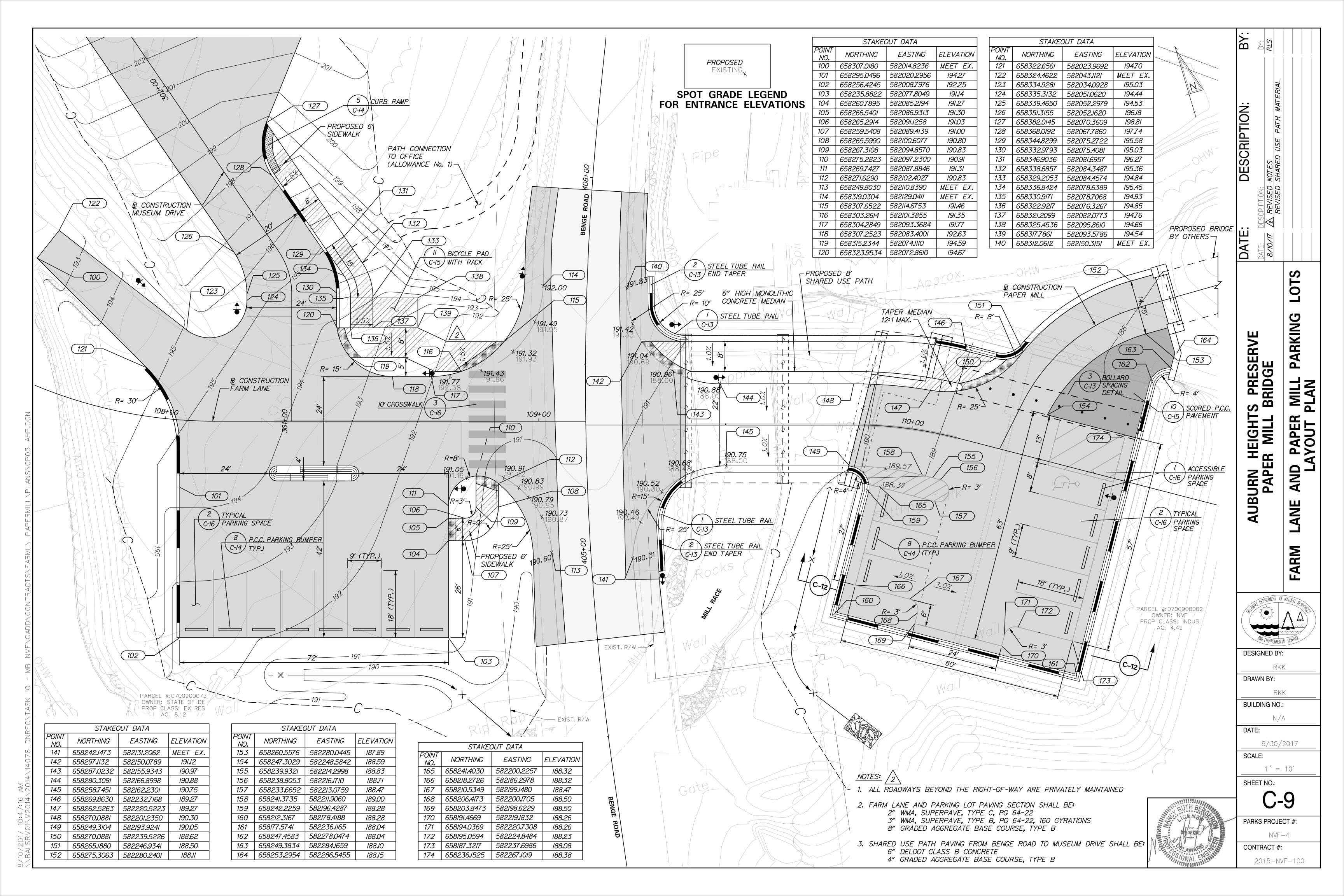
The following design elements shall not be altered by the Contractor's Design: Horizontal Alignment Vertical Profile Typical Section

END OF SECTION









- 2. REVIEW AND APPROVAL OF THE SEDIMENT AND STORMWATER MANAGEMENT PLAN SHALL NOT RELIEVE THE CONTRACTOR FROM HIS OR HER RESPONSIBILITIES FOR COMPLIANCE WITH THE REQUIREMENTS OF THE SEDIMENT AND STORMWATER MANAGEMENT REGULATIONS, NOR SHALL IT RELIEVE THE CONTRACTOR FROM FRORS OR OMISSIONS IN THE APPROVED PLAN.
- 3. IF THE APPROVED PLAN NEEDS TO BE MODIFIED, ADDITIONAL SEDIMENT AND STORMWATER MEASURES MAY BE REQUIRED AS DEEMED NECESSARY BY DNREC.
- 4. FOLLOWING SOIL DISTURBANCE OR REDISTURBANCE OF THE STAGING AREA, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN FOURTEEN (14) CALENDAR DAY AS TO THE SURFACE OF ALL PERIMETER SEDIMENT CONTROLS AND SOIL STOCKPILES.
- 5. UPON COMPLETION OF CONSTRUCTION AND APPROVAL BY DNREC'S SEDIMENT & STORMWATER PROGRAM, THE OWNER SHALL BE RESPONSIBLE FOR FUTURE MAINTENANCE OF THE PERMANENT STORMWATER MANAGEMENT FACILITIES.
- 6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTENANCE AND REPAIR OF ALL EROSION AND SEDIMENT CONTROL DEVICES AND STORMWATER MANAGEMENT PRACTICES DURING ALL CONSTRUCTION ACTIVITIES.
- 7. DUST IS TO BE CONTROLLED EXCLUSIVELY THROUGH THE USE OF WATER. COSTS ASSOCIATED WITH THE FURNISHING AND APPLICATION OF WATER FOR DUST CONTROL SHALL BE INCLUDED IN THE BID.
- 8. IT IS INTENDED THAT MUD TRACKING BE ELIMINATED ON ALL ROADWAYS AND PARKING LOTS ADJOINING THE PROJECT. EACH POINT OF INGRESS AND EGRESS FROM THE PROJECT SHALL HAVE INSTALLED A STABILIZED CONSTRUCTION ENTRANCE (SCE). ALL PAVED SURFACES ADJOINING THE PROJECT LIMITS SHALL BE LEFT IN A BROOM CLEAN CONDITION AT THE END OF EACH WORKDAY.
- 9. STABILIZED CONSTRUCTION ENTRANCES SHALL BE TOP DRESSED WITH TWO (2) INCHES OF CLEAN DE NO. STONES WHEN THE VOIDS BECOME CLOGGED OR AS DIRECTED BY THE OWNER. IN THE EVENT THAT IT IS DETERMINED THAT VEHICLES ARE TRANSPORTING SEDIMENT FROM THE SITE, A WHEEL WASH SHALL BE INSTALLED TO CLEAN VEHICLES PRIOR TO THEIR EGRESS FROM THE SITE AT NO ADDITIONAL COST. THIS DETERMINATION SHALL BE MADE BY THE ON SITE CERTIFIED CONSTRUCTION REVIEWER (CCR) AND/OR BY THE OWNER.
- 10. PRIOR TO ANY CLEARING, INSTALLATION OF SEDIMENT CONTROL MEASURES, OR GRADING, THE CONTRACTOR SHALL SCHEDULE AND CONDUCT A PRE-CONSTRUCTION MEETING WITH THE AGENCY CONSTRUCTION SITE REVIEWER. THE LANDOWNER/DEVELOPER REPRESENTATIVE, SITE CONTRACTOR, AND A CERTIFIED CONSTRUCTION REVIEWER ARE REQUIRED TO BE IN ATTENDANCE AT THE PRE-CONSTRUCTION MEETING; THE SITE DESIGNER IS RECOMMENDED TO ATTEND.
- 11. THE CONTRACTOR SHALL AT ALL TIMES PROTECT AGAINST SEDIMENT OR DEBRIS LADEN RUNOFF OR WIND FROM LEAVING THE SITE. PERIMETER CONTROLS SHOULD BE CHECKED DAILY AND ADJUSTED AND/OR REPAIRED TO FULLY CONTAIN AND CONTROL SEDIMENTATION ON THE SITE. ACCUMULATED SEDIMENT SHALL BE REMOVED WHEN IT HAS REACHED HALF OF THE EFFECTIVE CAPACITY OF THE CONTROL. IN ADDITION, THE CONTRACTOR MAY NEED TO ADJUST OR REPAIR MEASURES IN TIMES OF ADVERSE WEATHER CONDITIONS, AS NEEDED OR AS DIRECTED BY THE AGENCY CONSTRUCTION SITE REVIEWER.
- 12. THE CONTRACTOR SHALL NOTIFY THE PERSON RESPONSIBLE FOR STORMWATER SYSTEM CONSTRUCTION REVIEW AT LEAST THREE (3) DAYS PRIOR TO THE START OF THE STORMWATER SYSTEM CONSTRUCTION; STORMWATER FACILITIES MUST BE REVIEWED THROUGHOUT THEIR CONSTRUCTION.
- 13. STOCKPILE TOPSOIL AND EXCAVATED SUBSOILS. STOCKPILES SHALL BE SURROUNDED WITH A PERIMETER CONTROL, LOCATED ON LAND WITH SLIGHT TO NO SLOPE, AND STABILIZED ONCE INACTIVE.
- 14. SOILS WITHIN THE PROJECT AREA CONSIST OF: HATBORO-CODORUS COMPLEX (Hw), HYDROLOGIC SOIL GROUP B/D AND DELANCO-CODORUS-HATBORO COMPLEX (DcB), HYDROLOGIC SOIL GROUP C.
- 15. TOTAL DISTURBED AREA: 2.28 ACRES. TOTAL VOLUME OF SPOILS: APPROX. 3,000 C.Y.
 FOR THE COMPLETE LIMIT OF DISTURBANCE AREA, REFER TO THE CONSTRUCTION PLANS, SHEETS
 C-4 THROUGH C-5.
- 16. FOR EROSION AND SEDIMENT CONTROLS, SEE SHEETS ES-2 THROUGH ES-3.
- 17. ALL PERIMETER CONTROLS ARE TO BE REVIEWED BY THE AGENCY CONSTRUCTION SITE REVIEWER AND APPROVED PRIOR TO PROCEEDING WITH FURTHER SITE DISTURBANCE OR CONSTRUCTION.
- 18. EROSION AND SEDIMENT CONTROL DEVICES SHOULD BE REMOVED ONLY AFTER WORK IN AN AREA HAS BEEN COMPLETED AND STABILIZED, WITH WRITTEN APPROVAL FROM THE AGENCY CONSTRUCTION SITE REVIEWER.
- 19. PRIOR TO COMMENCING A NEW PHASE OF CONSTRUCTION, THE CONTRACTOR SHALL RECEIVE APPROVAL FROM THE AGENCY CONSTRUCTION SITE REVIEWER THAT THE PREVIOUS PHASE HAS BEEN SUFFICIENTLY STABILIZED.
- 20. THE TERMINATION OF THE CONSTRUCTION GENERAL PERMIT WILL REQUIRE SUBMISSION AND ACCEPTANCE OF THE POST CONSTRUCTION VERIFICATION DOCUMENTS, INCLUDING FINAL STABILIZATION THROUGHOUT THE SITE, ALL ELEMENTS OF THE SEDIMENT AND STORMWATER MANAGEMENT PLAN IMPLEMENTED, AND ACCEPTANCE OF THE FINAL OPERATION AND MAINTENANCE PLAN.

)	CONSTRUCTION PHASING & M.O.T		EROSION & SEDIMENT CONTROL		
	Ħ	BARRICADE, TYPE 3	SCE	STABILIZED CONSTRUCTION ENTRANCE	
)		CONCRETE SAFETY BARRIER - PORTABLE	——SF——	SILT FENCE	
	H	CONSTRUCTION WARNING SIGN LOCATION	RSF	SILT FENCE - REINFORCED	
	END ROAD WORK	CONSTRUCTION WARNING SIGN	——SAP——	SENSITIVE AREA PROTECTION	
	•	DRUM - TRAFFIC CONTROL	<u>CFL</u>	COMPOST FILTER LOG	
)			ROP-1	RIPRAP OUTLET PROTECTION, TYPE 1	

CONSTRUCTION PHASING PROJECT NOTES

ACCESS THROUGH THE CONSTRUCTION SITE

THE CONTRACTOR SHALL MAINTAIN ACCESS TO THE EXISTING RESIDENCES ON FARM LANE AT ALL TIMES THROUGHOUT THE LIFE OF THE PROJECT. THE CONTRACTOR SHALL MAINTAIN EMERGENCY ACCESS THROUGH THE WORK ZONE DURING EACH PHASE OF CONSTRUCTION. THE PROJECT SITE SHALL BE CLOSED TO PEDESTRIANS THROUGHOUT THE LIFE OF THE PROJECT.

WORK AREA ADJACENT TO BENGE ROAD

WHEN WORKING ADJACENT TO BENGE ROAD THE CONTRACTOR SHALL UTILIZE FLAGGERS IN ACCORDANCE WITH THE DELAWARE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) TA 10. THE CONTRACTOR SHALL EXCAVATE ONLY AS MUCH AS CAN BE CONSTRUCTED TO THE GABC COURSE AT THE END OF EACH WORK DAY. LONGITUDINAL DROP OFFS SHALL BE CORRECTED AT THE END OF EACH WORK DAY IN ACCORDANCE WITH TABLE 6G-1 OF THE DELAWARE MUTCD.

BRIDGE INSTALLATION - WORKIN BRIDGES

THE CONTRACTOR SHALL NOTE THAT FURNISHING AND INSTALLING PAPER
MILL BRIDGE OVER RED CLAY CREEK SHALL BE BY WORKIN BRIDGES UNDER SEPARATE
CONTRACT AND IS NOT INCLUDED IN THIS CONTRACT. THE CONTRACTOR SHALL BE RESPONSIBLE
FOR THE CONSTRUCTION OF THE ABUTMENTS AND SCOUR PROTECTION FOR THE NEW STRUCTURE
AS SHOWN IN THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL ANTICIPATE AND INCLUDE
AN UNINTERRUPTED 45 CALENDAR DAY PERIOD FOR THE INSTALLATION OF THE BRIDGE. THE
CONTRACTOR SHALL PROVIDE A MINIMUM 30 DAY NOTICE TO WORKIN BRIDGES AND THE OWNER
AS TO THE EXACT DATE WHEN THE ABUTMENTS AND WORK SITE SHALL BE AVAILABLE FOR THE
INSTALLATION OF THE BRIDGE. THE CONTRACTOR SHALL COORDINATE ONGOING CONSTRUCTION ACTIVITIES
WITH WORKIN BRIDGES SUCH AS TO NOT INTERRUPT OR DELAY THE INSTALLATION OF THE BRIDGE.
ACCESS TO THE BRIDGE CONSTRUCTION STAGING AREA SHALL BE MAINTAINED AT ALL TIMES.

CONTACT INFORMATION:

WORKIN BRIDGES - JULIE BOWERS 641.260.1262 JBOWERS@GMAIL.COM BACH STEEL - NELS RAYNOR 517.581.6243 NELS@BACHSTEEL.COM

STAGING / STOCKPILE AREAS

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE INSTALLATION, MAINTENANCE AND REPAIR/REPLACEMENT OF ALL EROSION AND SEDIMENT CONTROL DEVICES REQUIRED FOR THE INSTALLATION OF THE BRIDGES INCLUDING THE STAGING AREA SHOWN ON THE PLANS AS WORKIN BRIDGES' CONSTRUCTION STAGING AREA. ALL COST ASSOCIATED WITH THE INSTALLATION, MAINTENANCE AND REPAIR/REPLACEMENT OF THESE DEVICES SHALL BE INCLUDED IN THE CONTRACTOR'S LUMP SUM BID. NO ADDITIONAL PAYMENT SHALL BE MADE BY THE ORIGINAL CONDITION. ALL COSTS ASSOCIATED WITH THE RESTORING THESE AREA ARE TO BE INCLUDED IN THE CONTRACTOR'S LUMP SUM BID. NO ADDITIONAL PAYMENT SHALL BE MADE BY THE OWNER.

ACCESS TO PAPER MILL BRIDGE EAST ABUTMENT

THE CONTRACTOR SHALL USE THE HIPARCEL TRAIL TO ACCESS THE STAGING AREA NEAR
THE EAST ABUTMENT FOR PAPER MILL BRIDGE, THE ENTRANCE IS LOCATED OFF OF YORKLYN
ROAD, UPON COMPLETION OF THE PROJECT, INCLUDING THE INSTALLATION OF THE PAPER MILL
BRIDGE BY WORKIN BRIDGES, THE CONTRACTOR SHALL REPAIR THE EXISTING TRAIL AT THE
DIRECTION OF THE OWNER. THE COST FOR REPAIRS SHALL BE PAID FOR FROM ALLOWANCE
NO. 5 - TRAIL REPAIRS.

BENGE ROAD PARKING LOT

THE CONTRACTOR SHALL NOTE THE EXISTING PARK OFFICE MAY BE UNDER RENOVATION BY OTHERS UNDER SEPARATE CONTRACT DURING THE TIME OF THIS PROJECT. THE EXISTING STONE LOT NORTH OF THE OFFICE SHALL BE USED BY OTHERS DURING THE RENOVATION AND SHALL NOT BE AVAILABLE UNTIL THE RENOVATIONS ARE COMPLETE. THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION OF THE PROPOSED PARKING LOT IMPROVEMENTS INCLUDING THE PROPOSED SIDEWALK WITH THE CONTRACTOR OF THE RENOVATIONS AND SCHEDULE THE CONSTRUCTION OF THE IMPROVEMENTS ACCORDINGLY.

WORKING IN WATERWAYS

THE CONTRACTOR SHALL NOTIFY THE OWNER AND THE BOG TURTLE SURVEYOR 10 DAYS IN ADVANCE OF DEWATERING THE MILL RACE.

THE CONTRACTOR SHALL MONITOR THE WATER LEVELS AND COORDINATE WITH THE BOG TURTLE SURVEYOR. THE BOG TURTLE SURVEYOR SHALL MONITOR THE MILL RACE BELOW WHERE THE SAND BAGS DIVERSION IS PLACED TO THE MILL RACE OUTFALL ONCE THE WATER LEVEL IS APPROXIMATELY THREE INCHES IN DEPTH AND EACH DAY THEREAFTER UNTIL THE MILL RACE IS COMPLETELY DRY, TO ENSURE ANY TURTLES THAT MIGHT BE IN THE DRYING MILL RACE ARE NOT EXPOSED AND CAN BE MOVED TO SAFETY.

NO EQUIPMENT SHALL BE MOVED INTO THE MILL RACE UNTIL THE BED OF THE MILL RACE IS COMPLETELY DRY, ALL SURVEYS ARE COMPLETE AND THE BOG TURTLE SURVEYOR HAS RELEASED THE AREA FOR CONSTRUCTION.

__CLEARING_AND_GRUBBING_

THE LIMITS OF CLEARING AND GRUBBING SHALL BE TO THE LIMITS OF DISTURBANCE (LOD), INCLUDING THE STAGING AREA. THE COSTS ASSOCIATED WITH CLEARING AND GRUBBING ARE TO BE INCLUDED IN THE BASE BID.

TEMPORARY ACCESS ROAD /2

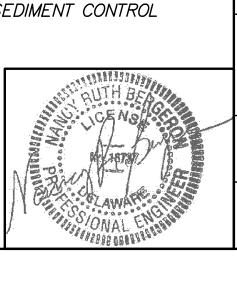
THE CONTRACTOR SHALL CONSTRUCT THE TEMPORARY ACCESS ROAD AS SHOWN ON THE PLANS.
UPON COMPLETION OF BRIDGE CONSTRUCTION, THE ACCESS ROAD SHALL BE REMOVED AND THE AREA
BACKFILLED, SEEDED AND MULCHED.

SUGGESTED CONSTRUCTION SEQUENCING NOTES- PHASE 1

- 1. NOTIFY THE DNREC SEDIMENT AND STORMWATER MANAGEMENT PROGRAM IN WRITING AT LEAST FIVE (5) DAYS PRIOR TO THE START OF CONSTRUCTION. FAILURE TO DO SO CONSTITUTES A VIOLATION OF THE APPROVED SEDIMENT AND STORMWATER MANAGEMENT PLAN.
- 2. INSTALL ADVANCED WARNING SIGNS AS SHOWN ON THE PLANS, SIGNS SHOWN ON PLAN ARE FOR WORK IN THE LOCATION OF FARM LANE AND THE PAPER MILL BRIDGE PARKING LOT, THE CONTRACTOR SHALL ADJUST SIGN LOCATIONS AS REQUIRED TO CONSTRUCT PARKING LOT IMPROVEMENTS ON BENGE ROAD NORTH OF THE OFFICE.
- 3. INSTALL EROSION AND SEDIMENT CONTROL DEVICES AS SHOWN ON THE PLANS. MARK THE LIMITS OF SENSITIVE AREAS, SUCH AS PRESERVED TREES, INFILTRATION AREAS, WETLANDS, AND OTHER SECTIONS THAT ARE NOT TO BE DISTURBED WITH A PHYSICAL BARRIER.
- 4. ONCE THE EROSION AND SEDIMENT PERIMETER CONTROLS ARE SET, THE CONTRACTOR SHALL SCHEDULE A PERIMETER CONTROL REVIEW WITH THE AGENCY CONSTRUCTION SITE REVIEWER. ALL PERIMETER CONTROLS ARE TO BE REVIEWED BY THE AGENCY CONSTRUCTION SITE REVIEWER AND APPROVED PRIOR TO PROCEEDING WITH FURTHER SITE DISTURBANCE OR CONSTRUCTION.
- 5. CLEAR AND GRUB AND REMOVE DEBRIS WITHIN THE PROJECT AREA AS SHOWN ON THE PLANS.
- 6. UTILIZE DELAWARE MUTCD TA-10 WHILE WORKING ADJACENT TO BENGE ROAD.
- 7. INSTALL TEMPORARY ROAD TO ACCESS PAPER MILL WEST ABUTMENT.
- 8. INSTALL SUPPORT OF EXCAVATION AS REQUIRED TO CONSTRUCT THE PROPOSED BRIDGE ABUTMENTS.
 THE SUPPORT OF EXCAVATION SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN
 THE STATE OF DELAWARE. WORKING DRAWINGS AND CALCULATIONS SHALL BE PROVIDED TO THE
 OWNER FOR REVIEW AND APPROVAL.
- 9. CONSTRUCT ABUTMENTS TO THE BEAM SEAT & CONSTRUCT WINGWALLS AS SHOWN ON PLANS.
- 10. REMOVE SUPPORT OF EXCAVATION.
- 11. INSTALL RIPRAP SCOUR PROTECTION.
- 12. RELEASE BRIDGE ABUTMENTS TO WORKIN' BRIDGES FOR INSTALLATION OF THE BRIDGES. SEE CONSTRUCTION PHASING PROJECT NOTES THIS SHEET.
- 13. INSTALL SAND BAG DIVERSION AT THE HEAD OF THE MILL RACE AND DEWATER THE EXISTING MILL RACE. SEE CONSTRUCTION PHASING PROJECT- WORKING IN WATERWAYS, THIS SHEET.
- 14. INSTALL THE PAPER MILL DECKOVER STRUCTURE.
- 15. CONSTRUCT PARKING AREA OVER EXISTING SETTLING TANK TO TOP OF GABC.
- 16. CONSTRUCT BENGE ROAD PARKING LOT IN ACCORDANCE WITH OWNER PREPARED DRAWINGS. SEE CONSTRUCTION PHASING PROJECT NOTES- BENGE ROAD PARKING LOT, THIS SHEET.
- 17. CONSTRUCT PATH CONNECTION FROM OFFICE TO PROVIDE PEDESTRIAN ACCESS TO THE EXISTING RESIDENCES ALONG BENGE ROAD DURING PHASE 2 CONSTRUCTION ACTIVITIES.
- 18. STABILIZE WORK AREA IN ACCORDANCE WITH THE DELAWARE EROSION AND SEDIMENT CONTROL HANDBOOK.
- 19. REMOVE EROSION AND SEDIMENT CONTROL DEVICES NOT REQUIRED FOR FUTURE PHASES.

SUGGESTED CONSTRUCTION SEQUENCING NOTES- PHASE 2

- . MAINTAIN EROSION AND SEDIMENT CONTROL DEVICES FROM PREVIOUS PHASE AND INSTALL ADDITIONAL EROSION AND SEDIMENT DEVICES REQUIRED FOR THIS PHASE.
- 2. INSTALL TEMPORARY TRAFFIC CONTROL DEVICES AS SHOWN ON THE PLANS.
- 3. CLOSE FARM LANE TO PUBLIC ACCESS. CLOSE MUSEUM DRIVE TO ALL TRAFFIC.
- 4. CONSTRUCT REMAINING PORTION OF OFFICE PATH CONNECTION. UTILIZE THE NEWLY CONSTRUCTED OFFICE PARKING LOT AND PATH FOR PEDESTRIAN ACCESS TO EXISTING RESIDENCES ALONG BENGE ROAD.
- 5. MAINTAIN LOCAL ACCESS TO FARM LANE AT ALL TIMES.
- 6. DEMO BUILDINGS AS SHOWN ON THE DEMOLITION PLAN
- 7. ROUGH GRADE WORK AREA.
- 8. INSTALL UTILITY SLEEVES TO THE FUTURE BUILDING.
- 9. CONSTRUCT IMPROVEMENTS AS SHOWN ON PLANS.
- 10. UPON COMPLETION OF BRIDGE ERECTION CONSTRUCT REMAINING BRIDGE ELEMENTS.
- 11. UPON COMPLETION OF REMAINING BRIDGE ELEMENTS, CONSTRUCT BOARDWALK CONNECTION TO HI PARCEL TRAIL.
- 12. REMOVE TEMPORARY ACCESS ROAD TO PAPER MILL WEST ABUTMENT UPON THE COMPLETION OF THE PAPER MILL BRIDGE AND RESTORE THE AREA TO IT'S ORIGINAL GRADES AND CONDITION.
- 13. CONSTRUCT REMAINING IMPROVEMENTS EAST OF BENGE ROAD.
- 14. STABILIZE WORK AREA IN ACCORDANCE WITH THE DELAWARE EROSION AND SEDIMENT CONTROL HANDBOOK.
- 15. REMOVE EROSION AND SEDIMENT CONTROL DEVICES AFTER APPROVAL BY DNREC'S SEDIMENT AND STORMWATER PROGRAM.



DESCRIPTION:

BATE:

BATON:

ALENISED AND ADDED NOTES

BY

ROSION AND OTES

AUBURN HEIGHTS PRESERVE PAPER MILL BRIDGE ONSTRUCTION PHASING & EROSI SEDIMENT CONTROL NOTES

AND ENVIRONMENTAL CONTROL

DESIGNED BY:

RKK

DRAWN BY:

DATE:

BUILDING NO.:

N/A

6/30/2017

SCALE:

NOT TO SCALE

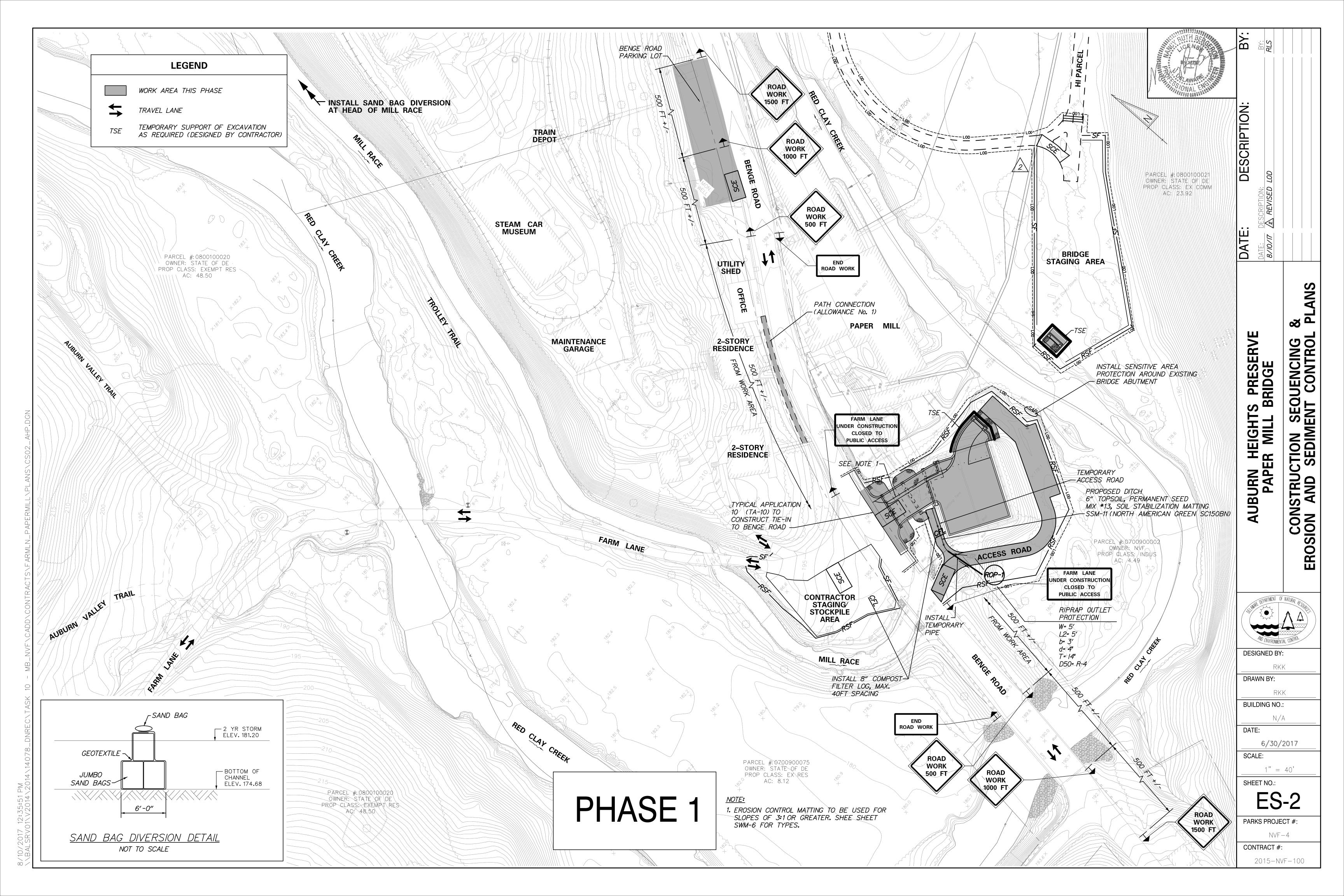
SHEET NO.:

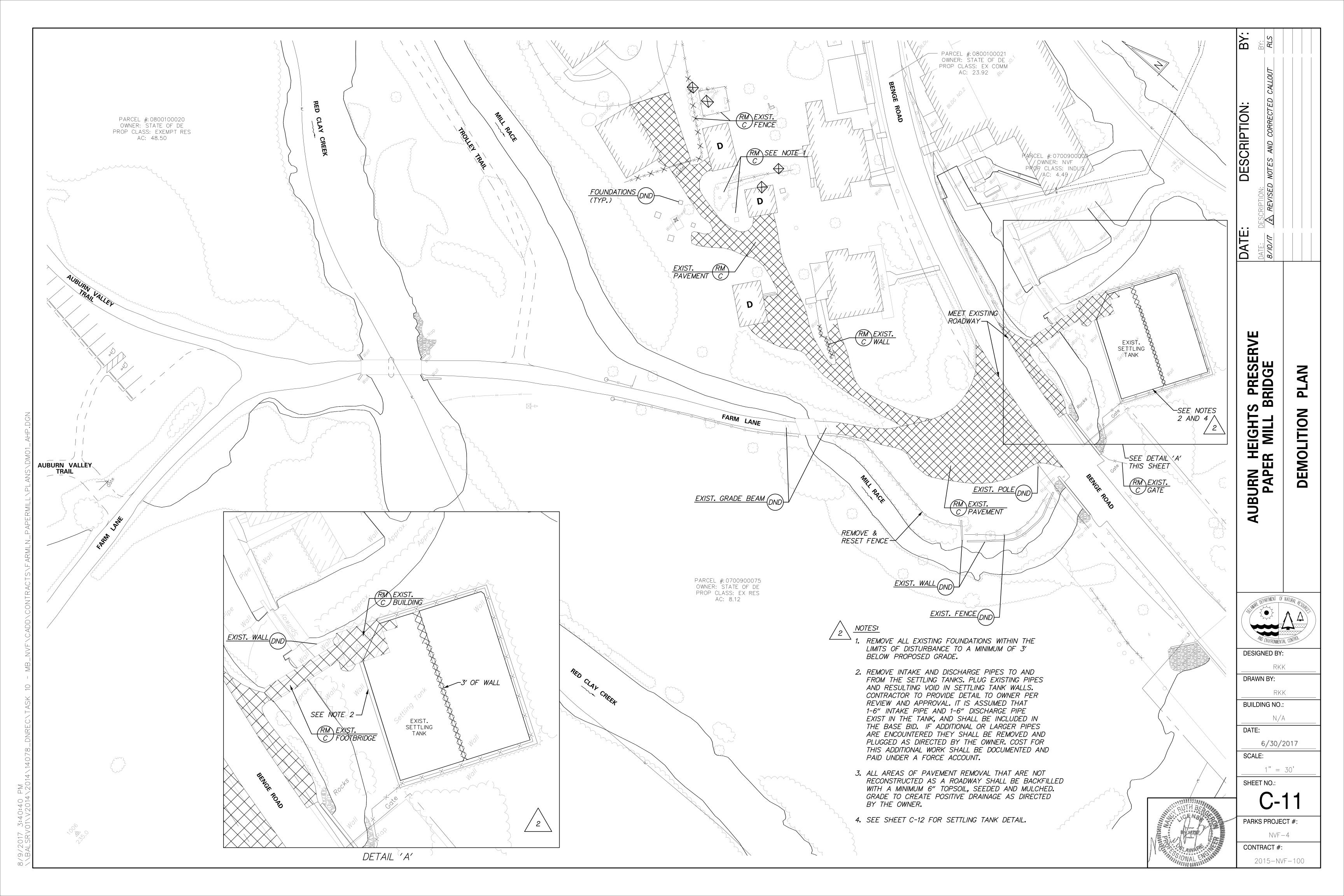
PARKS PROJECT #:

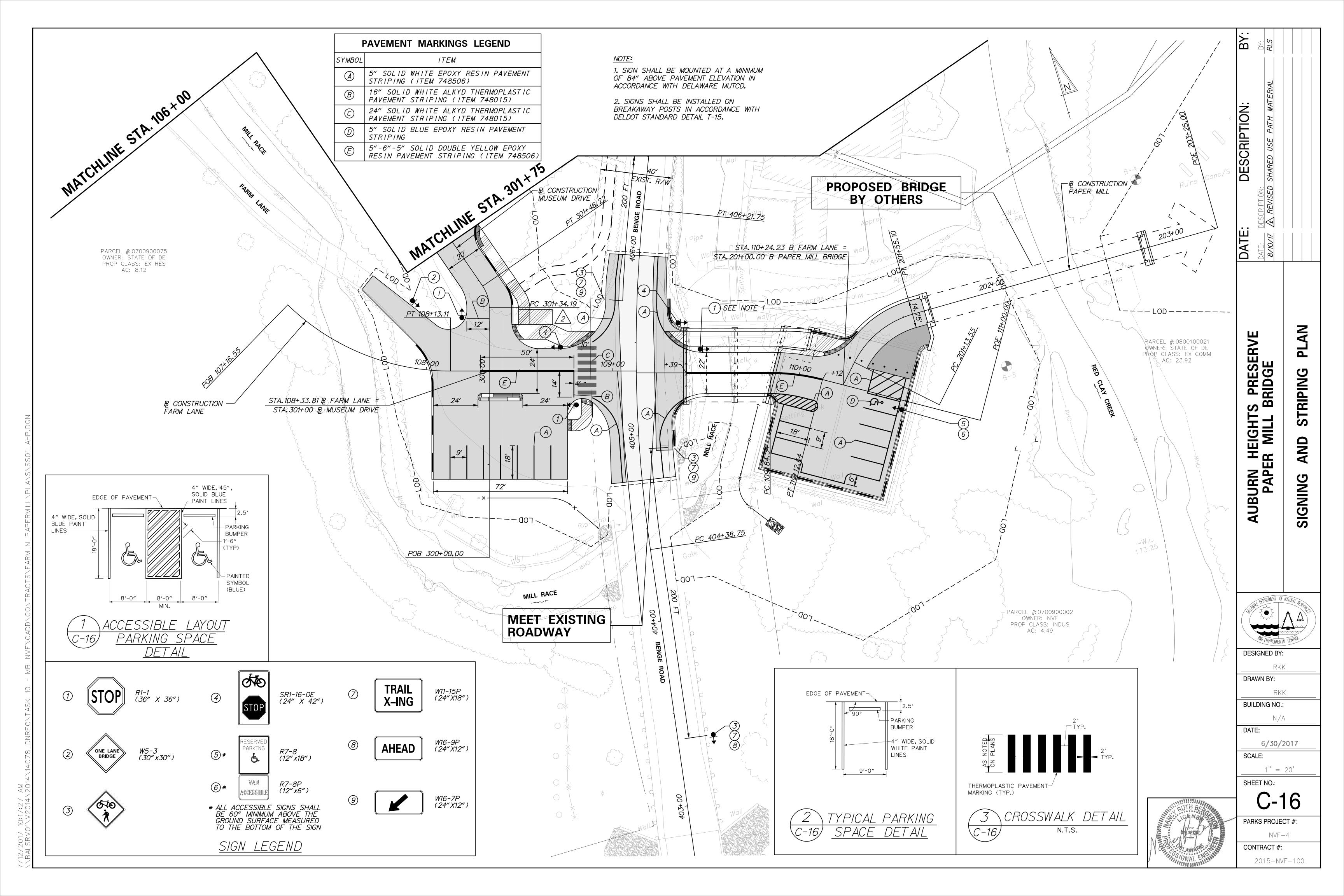
NVF-4

CONTRACT #:

2015-NVF-100







GEO-TECHNOLOGY ASSOCIATES, INC.

GEOTECHNICAL AND ENVIRONMENTAL CONSULTANTS





June 10, 2016

Rummel Klepper & Kahl, LLP 81 Mosher Street Baltimore, Maryland 21217

Attn: Mr. Robert Schaffer

Re: Report of Geotechnical Exploration

NVF Flood Plain Mitigation and Culverts

New Castle County, Delaware

Dear Mr. Schaffer:

In accordance with our agreement, Geo-Technology Associates, Inc. (GTA) has performed a geotechnical exploration for the proposed flood plain mitigation site, culverts, and bridge proposed along the Red Clay Creek at the former NVF plant and Auburn Heights in New Castle County, Delaware. The subsurface exploration consisted of performing Standard Penetration Test (SPT) borings at the proposed locations of the mitigation area, culverts, and bridge. The results of the subsurface exploration and laboratory testing are presented in this report.

We appreciate the opportunity to be of assistance on this project. Should you have questions or require additional information, please do not hesitate to contact our office.

Sincerely,

GEO-TECHNOLOGY ASSOCIATES, INC.

FOR

Edwin Williams

Associate

Christopher M. Reith, P.E.

Principal

SJ/CMR/amd 160621 No. 9618

No. 9618

RELAWARE

NO. 9618



REPORT OF GEOTECHNICAL EXPLORATION

NVF FLOOD PLAIN MITIGATION AND CULVERTS

New Castle County, Delaware

June 2016

Prepared For:

RUMMEL KLEPPER & KAHL, LLP

81 Mosher Street Baltimore, Maryland 21217

Attn: Mr. Robert Schaffer

Prepared By:

GEO-TECHNOLOGY ASSOCIATES, INC.

Geotechnical and Environmental Consultants 18 Boulden Circle, Suite 36 New Castle, Delaware 19720 (302) 326-2100

GTA Job No: 160621

TABLE OF CONTENTS

SUBJECT	PAGE
INTRODUCTION	1
SITE GEOLOGY	1
Geologic Maps	1
Soil Survey	2
FIELD EXPLORATION	2
LABORATORY TESTING	4
PRESENTATION OF BORING DATA	4
LIMITATIONS	4
ASFE - Important Information About Your Geotechnical Report	
APPENDICES	
Appendix A - Figures Figure A-1: Boring Location Plan	
Appendix B - Test Boring Data Notes for Exploration Logs Boring Logs (B-1 through B-11)	
Appendix C – Laboratory Test Data Table C-1 – Summary of Laboratory Test Data Particle Size Distribution Reports (12 Sheets)	

REPORT OF GEOTECHNICAL EXPLORATION

NVF FLOOD PLAIN MITIGATION AND CULVERTS NEW CASTLE COUNTY, DELAWARE JUNE 2016

INTRODUCTION

Geo-Technology Associates, Inc. (GTA) has completed a subsurface exploration for the proposed improvements at as part of the NVF Floodplain Mitigation and Culvert Replacement, located in New Castle County, Delaware. GTA understands that the proposed construction includes a floodplain/wetland mitigation area, the replacement and relocation of one culvert, the addition of a second culvert, and a new bridge. The flood plain mitigation site is located north of the Auburn Heights Mansion between the Red Clay Creek and millrace, the culvert replacement/realignment will be at Farm Lane where the road crosses over the mill race, the Paper Mill culvert/bridge will cross the mill race, and the new bridge will cross Red Clay Creek at the former NVF plant. Included in the field explorations were 11 Standard Penetration Test borings drilled to depths of 10 to 37 feet below the existing ground surface. Limited laboratory testing was performed on split spoon samples recovered from the borings to characterize general subsurface conditions. The results of the field and laboratory testing are detailed herein.

SITE GEOLOGY

Geologic Maps

According to the *Bedrock Geologic Map of the Piedmont of Delaware & Adjacent Pennsylvania* (2000), prepared by the Delaware Geological Survey (DGS), the area is situated in the Piedmont Geographic Province. The map indicates that most of the borings were drilled in the Precambrian Age Baltimore Gneiss. The borings for the bridge at the former NVF Plant are near the geologic contact between the Baltimore Gneiss and the Cambrian-Ordovician Age Cockeysville Marble.

The Geologic Map of New Castle County (2005) indicates that some areas adjacent to the Red Clay Creek are underlain by Holocene Age alluvial deposits. These materials are described as being in the stream beds and adjacent flood plains.

Soil Survey

According to the U.S. Department of Agriculture (USDA), National Resources Conservation Service (NRCS) Web Soil Survey (2015), the soils underlying the wetland mitigation site and culverts are generally mapped as the Hatboro-Codorus complex. The Hatboro frequently flooded soil series is characterized as a flood plain soil with poor drainage. Lithic bedrock is indicated to be at a depth of 60 to 118 inches with a water table depth of 0 to 6 inches. The Codorus occasionally flooded soil series is characterized as a flood plain soil that is moderately well drained. Lithic bedrock is indicated to be at a depth of more than 80 inches with a water table depth of 18 to 36 inches. The soils underlying the uplands portion of the culvert areas are mapped as the Delanco-Hatboro-Codorus complex, characterized as an alluvial soil which is moderately well drained. The depth to water table is mapped at about 20 to 40 inches with lithic bedrock mapped at 60 to 118 inches deep. The soils underlying the NVF bridge are mapped as Urban Land bedrock substratum. No soils description is given for this series, but the depth to lithic bedrock is listed as 40 to 72 inches. Refer to the publications for more detailed information.

FIELD EXPLORATION

The subsurface exploration consisted of drilling 11 Standard Penetration Test (SPT) borings from April 25 through April 29, 2016. The borings were field located by GTA using hand held GPS equipment and coordinates supplied by RK&K. Elevations were not provided and the contours on the provided plans were not labelled. A few of the borings were off-set from the specified locations by a few feet due to accessibility issues. The approximate locations are indicated on the Test Boring Location Plan included in Appendix A.

The borings were performed by GTA using a CME 550X or CME 55 drill rig. Soil borings were advanced using hollow stem augers and an automatic hammer to depths ranging from 10 to 37 feet below existing ground surface. Standard Penetration Testing was performed in the boreholes,

with soil samples generally obtained continuously in the upper 10 feet and then approximately 5-foot intervals thereafter. Standard Penetration Testing involves driving a 2-inch O.D., 1³/₈-inch I.D. split-spoon sampler with a 140-pound hammer free falling 30 inches. The number of blows required to drive the sampler was recorded in intervals of 6 inches. The total number of hammer blows required to drive the sampler from the 6 to 18-inch interval is the SPT N-value. Uncorrected SPT blow counts have been recorded on the borings logs.

Split spoon samples retrieved from the test borings were returned to GTA's laboratory for visual classification by engineering personnel and limited laboratory testing. Descriptions provided on the logs are based on visual classifications by a GTA engineer in accordance with ASTM D2488 (Unified Soil Classification System), supplemented by available laboratory test results. The field sampling information is included on the test boring logs contained in Appendix B. Descriptions of the soil classification system are also included in Appendix B.

Topsoil thicknesses at the boring locations generally ranged from 1 to 8 inches, except at Boring B-1 which was in the dirt/gravel drive lane.

The depth to groundwater was noted during the drilling operations and groundwater levels were measured at the completion of drilling and in general, approximately 24 to 48 hours after completion. The test borings were backfilled after the 24 to 48 hour water readings were obtained except in areas with safety concerns or on the last day of drilling, where the borings were backfilled upon completion. The depth to the bottom of each borehole was also measured after the removal of the augers to measure the depth of collapse or cave-in.

Groundwater was encountered in the test borings during drilling at depths ranging from approximately 6 to 16 ft bgs. The observed water levels are unstabilized readings and actual levels are likely slightly higher. Groundwater was observed at depths ranging from approximately $3\frac{1}{2}$ to 7 ft bgs after a period of 24 to 48 hours. The water table will fluctuate several feet due to variations in precipitation and surface runoff and is likely commensurate with the adjacent creek.

LABORATORY TESTING

The soil samples retrieved from the test borings were brought to GTA's AASHTO Materials Reference Laboratory (AMRL) accredited (R18) laboratory for review by laboratory personnel and limited laboratory testing. Index testing was performed on the samples obtained from the borings to aid in classification of the soils. The laboratory testing consisted of determining the natural moisture content, the particle-size distribution, and the Atterberg Limits of the soil samples recovered from the split spoon sampler. Results of the testing for the samples are summarized in Table C-1, in Appendix C. The particle size distribution test results are also included in Appendix C.

PRESENTATION OF BORING DATA

The test boring information is contained on the final boring logs included in Appendix B. The test boring information on the final logs represents an interpretation of the field logs and includes modifications based on laboratory soil testing and comparison by visual examination during review of the recovered samples.

LIMITATIONS

This report has been prepared for the exclusive use of RK&K in accordance with generally accepted engineering practice. No warranty, express or implied, is made. Use and reproduction of this report by any other person without the expressed written permission of GTA is unauthorized, and such use is at the sole risk of the user.

The borings indicate soil conditions only at specific locations and times and only to the depths penetrated. They do not necessarily reflect strata variations that may exist between the test locations. Geo-Technology Associates, Inc. is not responsible for any claims, damages, or liability associated with interpretation of subsurface data or reuse of the subsurface data or engineering analysis without the expressed written authorization of Geo-Technology Associates, Inc.

151975

GEO-TECHNOLOGY ASSOCIATES, INC.

Important Information about This

Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

Geotechnical Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a civil engineer may not fulfill the needs of a constructor — a construction contractor — or even another civil engineer. Because each geotechnical- engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. No one except you should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one* — *not even you* — should apply this report for any purpose or project except the one originally contemplated.

Read the Full Report

Serious problems have occurred because those relying on a geotechnical-engineering report did not read it all. Do not rely on an executive summary. Do not read selected elements only.

Geotechnical Engineers Base Each Report on a Unique Set of Project-Specific Factors

Geotechnical engineers consider many unique, project-specific factors when establishing the scope of a study. Typical factors include: the client's goals, objectives, and risk-management preferences; the general nature of the structure involved, its size, and configuration; the location of the structure on the site; and other planned or existing site improvements, such as access roads, parking lots, and underground utilities. Unless the geotechnical engineer who conducted the study specifically indicates otherwise, do not rely on a geotechnical-engineering report that was:

- not prepared for you;
- not prepared for your project;
- not prepared for the specific site explored; or
- completed before important project changes were made.

Typical changes that can erode the reliability of an existing geotechnical-engineering report include those that affect:

- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a lightindustrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes—even minor ones—and request an

assessment of their impact. Geotechnical engineers cannot accept responsibility or liability for problems that occur because their reports do not consider developments of which they were not informed.

Subsurface Conditions Can Change

A geotechnical-engineering report is based on conditions that existed at the time the geotechnical engineer performed the study. Do not rely on a geotechnical-engineering report whose adequacy may have been affected by: the passage of time; man-made events, such as construction on or adjacent to the site; or natural events, such as floods, droughts, earthquakes, or groundwater fluctuations. Contact the geotechnical engineer before applying this report to determine if it is still reliable. A minor amount of additional testing or analysis could prevent major problems.

Most Geotechnical Findings Are Professional Opinions

Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Geotechnical engineers review field and laboratory data and then apply their professional judgment to render an opinion about subsurface conditions throughout the site. Actual subsurface conditions may differ — sometimes significantly — from those indicated in your report. Retaining the geotechnical engineer who developed your report to provide geotechnical-construction observation is the most effective method of managing the risks associated with unanticipated conditions.

A Report's Recommendations Are Not Final

Do not overrely on the confirmation-dependent recommendations included in your report. Confirmation-dependent recommendations are not final, because geotechnical engineers develop them principally from judgment and opinion. Geotechnical engineers can finalize their recommendations only by observing actual subsurface conditions revealed during construction. The geotechnical engineer who developed your report cannot assume responsibility or liability for the report's confirmation-dependent recommendations if that engineer does not perform the geotechnical-construction observation required to confirm the recommendations' applicability.

A Geotechnical-Engineering Report Is Subject to Misinterpretation

Other design-team members' misinterpretation of geotechnical-engineering reports has resulted in costly

problems. Confront that risk by having your geotechnical engineer confer with appropriate members of the design team after submitting the report. Also retain your geotechnical engineer to review pertinent elements of the design team's plans and specifications. Constructors can also misinterpret a geotechnical-engineering report. Confront that risk by having your geotechnical engineer participate in prebid and preconstruction conferences, and by providing geotechnical construction observation.

Do Not Redraw the Engineer's Logs

Geotechnical engineers prepare final boring and testing logs based upon their interpretation of field logs and laboratory data. To prevent errors or omissions, the logs included in a geotechnical-engineering report should *never* be redrawn for inclusion in architectural or other design drawings. Only photographic or electronic reproduction is acceptable, *but recognize that separating logs from the report can elevate risk*.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can make constructors liable for unanticipated subsurface conditions by limiting what they provide for bid preparation. To help prevent costly problems, give constructors the complete geotechnical-engineering report, but preface it with a clearly written letter of transmittal. In that letter, advise constructors that the report was not prepared for purposes of bid development and that the report's accuracy is limited; encourage them to confer with the geotechnical engineer who prepared the report (a modest fee may be required) and/ or to conduct additional study to obtain the specific types of information they need or prefer. A prebid conference can also be valuable. *Be sure constructors have sufficient time* to perform additional study. Only then might you be in a position to give constructors the best information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions.

Read Responsibility Provisions Closely

Some clients, design professionals, and constructors fail to recognize that geotechnical engineering is far less exact than other engineering disciplines. This lack of understanding has created unrealistic expectations that have led to disappointments, claims, and disputes. To help reduce the risk of such outcomes, geotechnical engineers commonly include a variety of explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help

others recognize their own responsibilities and risks. *Read these provisions closely*. Ask questions. Your geotechnical engineer should respond fully and frankly.

Environmental Concerns Are Not Covered

The equipment, techniques, and personnel used to perform an *environmental* study differ significantly from those used to perform a *geotechnical* study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated environmental problems have led to numerous project failures*. If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. *Do not rely on an environmental report prepared for someone else*.

Obtain Professional Assistance To Deal with Mold

Diverse strategies can be applied during building design, construction, operation, and maintenance to prevent significant amounts of mold from growing on indoor surfaces. To be effective, all such strategies should be devised for the express purpose of mold prevention, integrated into a comprehensive plan, and executed with diligent oversight by a professional mold-prevention consultant. Because just a small amount of water or moisture can lead to the development of severe mold infestations, many mold- prevention strategies focus on keeping building surfaces dry. While groundwater, water infiltration, and similar issues may have been addressed as part of the geotechnical- engineering study whose findings are conveyed in this report, the geotechnical engineer in charge of this project is not a mold prevention consultant; none of the services performed in connection with the geotechnical engineer's study were designed or conducted for the purpose of mold prevention. Proper implementation of the recommendations conveyed in this report will not of itself be sufficient to prevent mold from growing in or on the structure involved.

Rely, on Your GBC-Member Geotechnical Engineer for Additional Assistance

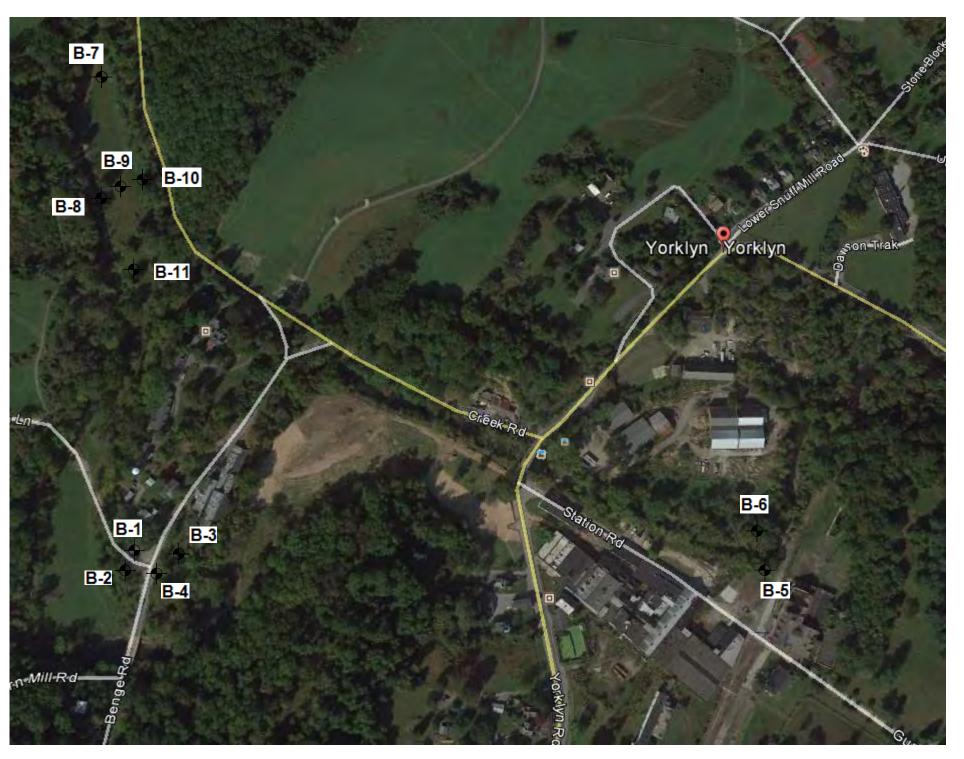
Membership in the Geotechnical Business Council of the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project. Confer with you GBC-Member geotechnical engineer for more information.



8811 Colesville Road/Suite G106, Silver Spring, MD 20910 Telephone: 301/565-2733 Facsimile: 301/589-2017 e-mail: info@geoprofessional.org www.geoprofessional.org

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APPENDIX A



JOB NUMBER.: 160621

> FIGURE: A-1

REVIEW BY: ΕW

SCALE:

DATE:

June 2016

NEW CASTLE COUNTY, DELAWARE

NVF Flood Plain Mitigation and Culverts **TEST BORING LOCATION PLAN**

GEO-TECHNOLOGY ASSOCIATES, INC. Geotechnical and Environmental Consultants 18 Boulden Circle, Suite 36
New Castle, Delaware 19720 (302) 326-2100
Fax (302) 326-2399

Notes: (1) Base drawing obtained from Google Earth
(2) Test Boring Location Plan should be read together with GTA Report Job No. 160621

(3) Indicates Test Boring location

APPENDIX B

NOTES FOR EXPLORATION LOGS

KEY TO USCS TERMINOLOGY AND GRAPHIC SYMBOLS

	SYMBOLS										
	MAJOR DIVISIONS (BASED UPON ASTM D 2488)										
	GRAVEL AND	CLEAN GRAVELS		GW							
COARSE - GRAINED	GRAVELY SOILS	(LESS THAN 5% PASSING THE NO. 200 SIEVE)		GP							
SOILS	MORE THAN 50% OF COARSE FRACTION RETAINED ON NO.	GRAVELS WITH FINES		GM							
	4 SIEVE	(MORE THAN 15% PASSING THE NO. 200 SIEVE)		GC							
	SAND AND	CLEAN SANDS		SW							
MORE THAN 50% OF MATERIAL IS LARGER THAN	SANDY SOILS	(LESS THAN 5% PASSING THE NO. 200 SIEVE)		SP							
NO. 200 SIEVE SIZE	MORE THAN 50% OF COARSE FRACTION	SANDS WITH FINES		SM							
	PASSING ON NO. 4 SIEVE		SC								
	SILTS	SILT OR CLAY (<15% RETAINED THE NO. 200 SIEVE)		ML							
FINE - GRAINED SOILS	AND CLAYS	SILT OR CLAY WITH SAND OR GRAVEL (15% TO 30% RETAINED THE NO. 200 SIEVE)		CL							
GOILG	LIQUID LIMIT LESS THAN 50		OL								
MORE THAN 50%	SILTS AND	SILT OR CLAY (<15% RETAINED THE NO. 200 SIEVE)		МН							
OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	CLAYS	SILT OR CLAY WITH SAND OR GRAVEL (15% TO 30% RETAINED THE NO. 200 SIEVE)		СН							
	LIQUID LIMIT GREATER THAN 50	. I SANDI SIL SILI SILI SILI SILI SILI SILI SIL									
	HIGHLY ORGANIC SOILS										

NOTE: DUAL SYMBOLS ARE USED TO INDICATE COARSE-GRAINED SOILS CONTAINING AN ESTIMATED 10% FINES BY VISUAL CLASSIFICATION OR WHEN THE SOIL HAS BETWEEN 5 AND 12 PERCENT FINES FROM LABORATORY TESTS; AND FOR FINE-GRAINED SOILS WHEN THE PLOT OF LIQUID LIMIT & PLASTICITY INDEX VALUES FALLS IN THE PLASTICITY CHART'S CROSSHATCHED AREA. RESULTS OF LABORATORY TESTING ARE USED TO SUPPLEMENT THE CLASSIFICATION OF THE SOILS BASED ON THE VISUAL-MANUAL PROCEDURES OF ASTM D2488.

ADDITIONAL TERMINOLOGY AND GRAPHIC SYMBOLS

ADDITIONAL DESIGNATION	DESCRI	GRAPHIC SYMBOLS	
	TOPSO	7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
	MAN-MAD		
	GLACIAL		
	COBBLES AND	0.0.0.0	
	DESCRIPTION	"N" VALUE	
RESIDUAL SOIL	HIGHLY WEATHERED ROCK	50 TO 50/1"	
DESIGNATION	PARTIALLY WEATHERED ROCK	MORE THAN 50 BLOWS FOR 1" PENETRATION, AUGER PENETRABLE	

COARSE-GRAINED SOILS (GRAVEL AND SAND)

DESIGNATION	BLOWS PER FOOT (BPF) "N"
VERY LOOSE	0 - 4
LOOSE	5 - 10
MEDIUM DENSE	11 - 30
DENSE	31 - 50
VERY DENSE	>50

NOTE: "N" VALUE DETERMINED AS PER ASTM D1586

FINE-GRAINED SOILS (SILT AND CLAY)

CONSISTENCY	BPF "N"
VERY SOFT	<2
SOFT	2 - 4
MEDIUM STIFF	5 - 8
STIFF	9 - 15
VERY STIFF	16 - 30
HARD	>30

NOTE: ADDITIONAL DESIGNATIONS TO ADVANCE SAMPLER INDICATED IN BLOW COUNT COLUMN: WOH = WEIGHT OF HAMMER WOR = WEIGHT OF ROD(S)

SAMPLE TYPE

DESIGNATION	SYMBOL
SPLIT-SPOON	S-
SHELBY TUBE	U-
ROCK CORE	R-

WATER DESIGNATION

DESCRIPTION	SYMBOL
ENCOUNTERED DURING DRILLING	\sqsubseteq
UPON COMPLETION OF DRILLING	T
24 HOURS AFTER COMPLETION	<u></u>

NOTE: WATER OBSERVATIONS WERE MADE AT THE TIME INDICATED. POROSITY OF SOIL STRATA, WEATHER CONDITIONS, SITE TOPOGRAPHY, ETC. MAY CAUSE WATER LEVEL CHANGES.

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft): Table 13.0

₩ Dry

BOC

PROJECT NO.: 160621

DATE:

4/26/16

4/26/16

PROJECT LOCATION: New Castle County, Delaware

CAVED (ft): In Auger

12.2

4/26/16

DATE STARTED: 4/26/2016

WATER ENCOUNTERED DURING DRILLING (ft) 13.0

GROUND SURFACE ELEVATION:

DRILLING CONTRACTOR: GTA

DATE COMPLETED: 4/26/2016

DATUM: Topo

EQUIPMENT: CME550X

DRILLING METHOD: Hollow Stem Auger SAMPLING METHOD: Split Spoon

DRILLER: D. Hans

LOGGED BY: T. Hill

CHECKED BY: C. Reith

SAN	<u> IPLIN</u>	<u>G METI</u>	HOD: Split	t Spo	on				CHECKED BY	C. Reith
SAMPLE NUMBER	SAMPLE DEPTH (ft.)	SAMPLE RECOVERY (in.)	SAMPLE BLOWS/6 inches	N (blows/ft.)	ELEVATION (ft.)	DEPTH (ft.)	nscs	GRAPHIC SYMBOL	DESCRIPTION	REMARKS
	0.0	.,			0.0	0-	SC		Brown, moist, Clayey SAND	Augered to 2.0 feet.
S-1	2.0	12	2-5-6-8	11	-2.0	-	SM		Brown and tan, moist, medium dense, Silty SAND, contains rock fragments	
S-2	4.0	5	2-5-7-8	12		5-			Same	
S-3	6.0	15	5-10-10-10	20		-			Dark brown, moist, medium dense, micaceous, Silty SAND	
S-4	8.0	17	4-14-12-17	26		- 10 -			Tan, moist, medium dense, Silty SAND, contains rock fragments	
						-				
S-5	13.0	1	50/2		-13.0	- -	HW		Dark brown, wet, very dense, Highly Weathered ROCK	▼
					-15.0	15 - -		7	Auger refusal at 15.0 feet.	
						-				
						20 -				
						-				
						25 -				
						- -				
						-				

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

LOG OF BORING NO. B-1

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft): DATE:

⊈ 6.7 **₹** 8.0_ 4/25/16

¥ 6.2

PROJECT NO.: 160621

PROJECT LOCATION: New Castle County, Delaware

CAVED (ft): In Auger 7.5

4/25/16 4/26/16 7.8

DATE STARTED: 4/25/2016

WATER ENCOUNTERED DURING DRILLING (ft) 8.0

DRILLING CONTRACTOR: GTA

DATE COMPLETED: 4/25/2016

GROUND SURFACE ELEVATION:

DATUM: Topo

DRILLER: D. Hans

EQUIPMENT: CME550X

DRILLING METHOD: Hollow Stem Auger

SAMPLING METHOD:	Split Spoon
	Opiil Opooli

LOGGED BY: T. Hill CHECKED BY: C. Reith

SAIV	IPLIN	GMEIF	IOD: Spli	t Spo	on				CHECKED BY	: C. Reith
SAMPLE	SAMPLE DEPTH (ft.)	SAMPLE RECOVERY (in.)	SAMPLE BLOWS/6 inches	N (blows/ff.)	ELEVATION (ft.)	DEPTH (ft.)	nscs	GRAPHIC SYMBOL	DESCRIPTION	REMARKS
S-1	0.0	12	1-1-1-2	2	0.0 -0.3	0 -	ML		Topsoil +/- 4 inches Brown, moist, soft, Sandy SILT	_
S-2	2.0	11	2-3-4-3	7	-2.0	-	SM		Brown, moist, loose, Silty SAND, contains rock fragments	
S-3	4.0	15	1-1-2-3	3		5 –			Brown, moist, very loose, Silty SAND	
S-4	6.0	21	3-3-6-6	9		-			Same, wet	<u>₹</u>
S-5	8.0	14	2-4-6-7	10		-			Same, medium dense	<u>.</u>
S-6	13.0	11	2-10-18-14	28		10 -			Brown and white, wet, medium dense, Silty SAND	
					-18.0	15 -	ыл.	\$ ∆ 7 ∧ ∧	Brown and white, wet, very dense, High Weathered	
S-7	18.0	10	24-50/4	50/4	-18.8	_	1100	ΧΔ	ROCK	_
						20-			Boring terminated at 18.8 feet.	

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

LOG OF BORING NO. B-2

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft): 10.0 10.0

PROJECT NO.: 160621

DATE: _ CAVED (ft): In Auger 10.9

4/26/16 4/26/16 4/28/16 8.5

PROJECT LOCATION: New Castle County, Delaware

WATER ENCOUNTERED DURING DRILLING (ft) 10.0

DRILLING CONTRACTOR: GTA

DATE COMPLETED: 4/26/2016

GROUND SURFACE ELEVATION:

DATUM: Topo

DRILLER: D. Hans

DATE STARTED: 4/26/2016

EQUIPMENT: CME550X

LOGGED BY: T. HIII DRILLING METHOD: Hollow Stem Auger CHECKED BY: C. Reith SAMPLING METHOD: Split Spoon

SAMPLE NUMBER	SAMPLE DEPTH (ft.)	SAMPLE RECOVERY (in.)	SAMPLE BLOWS/6 inches	N (blows/ft.)	ELEVATION (ft.)	DEPTH (ft.)	nscs	GRAPHIC SYMBOL	DESCRIPTION	REMARKS
S-1 S-2 S-3 S-4 S-5	0.0 2.0 4.0 6.0 8.0	14 13 16 15	1-2-3-3 3-3-2-3 1-2-2-3 2-2-2-3	5 5 4 4	0.0	0 - - 5 - -	FILL		Topsoil +/- 2 inches Brown, moist, loose, silty sand FILL, contains rock fragments Same Same Dark brown and gray, moist, very loose, clayey sand FILL, contains rock fragments Same, loose	▼
S-6 S-7	13.0	5	6-10-12-17 11-29-50/4		-13.0 -18.0 -19.3	15 — 			Brown, wet, medium dense, Silty SAND Brown, wet, very dense, Highly Weathered ROCK Boring terminated at 19.3 feet.	
						25 -				

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

LOG OF BORING NO. B-3

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft):

¥ 16.0

Ţ Dry

PROJECT NO .: 160621

PROJECT LOCATION: New Castle County, Delaware

DATE: _ CAVED (ft): In Auger

4/26/16 4/26/16 14.3

4/28/16 9.7

DATE STARTED: 4/26/2016

WATER ENCOUNTERED DURING DRILLING (ft) 16.0

GROUND SURFACE ELEVATION:

DRILLING CONTRACTOR: GTA

DATE COMPLETED: 4/26/2016

DATUM: Topo

DRILLER: D. Hans

EQUIPMENT: CME550X LOGGED BY: T. Hill

CHECKED BY: C. Reith

DRILLING METHOD: Hollow Stem Auger SAMPLING METHOD: Split Spoon

SAMPLE RECOVERY (in.) SAMPLE BLOWS/6 inches ELEVATION (ft. SAMPLE SAMPLE DEPTH (ft.) GRAPHIC SYMBOL N (blows/ft.) DEPTH (ft.) nscs REMARKS DESCRIPTION 0.0 Topsoil +/- 6 inches Augered to 2.0 -0.5 SC Brown, moist, Clayey SAND feet. Brown, moist, loose, Clayey SAND 3-3-3-3 6 S-1 2.0 12 -4.0 Brown, moist, medium dense, Silty SAND, contains rock SM fragments S-2 5 4.0 17 2-5-9-10 14 Same, brown and white S-3 6.0 18 8-8-13-17 21 -8.0 Brown and gray, moist, very dense, Highly Weathered HW 14-27-31-ROCK S-4 8.0 10 58 20 10 Same, brown S-5 11-50/5 50/5 13.0 8 15 Same S-6 18.0 6-18-27-34 45 17 -20.0 20 Boring terminated at 20.0 feet. 25

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

LOG OF BORING NO. B-4

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft): 16.0 DATE: _

¥ Dry 4/26/16 4/26/16

4/28/16 7.2

PROJECT NO.: 160621

PROJECT LOCATION: New Castle County, Delaware

CAVED (ft): In Auger

DATE COMPLETED: 4/26/2016 DRILLING CONTRACTOR: GTA

DATE STARTED: 4/26/2016

WATER ENCOUNTERED DURING DRILLING (ft) 16.0

GROUND SURFACE ELEVATION:

DATUM: Topo EQUIPMENT: CME550X

13.7

DRILLER: D. Hans DRILLING METHOD: Hollow Stem Auger

LOGGED BY: T. Hill

SAN	PLIN	G METH	OD: Spli		on			-	CHECKED BY	: C. Reith
SAMPLE	SAMPLE DEPTH (ft.)	SAMPLE RECOVERY (in.)	SAMPLE BLOWS/6 inches	N (blows/ft.)	ELEVATION (ft.)	DEPTH (ft.)	nscs	GRAPHIC SYMBOL	DESCRIPTION	REMARKS
	İ								DESCRIPTION	REWARKS
S-1	0.0	16	3-4-4-4	8	0.0 -0.1	0-	FILL		Topsoil +/- 1 inches Dark brown and black, moist, loose, silty sand FILL, contains rock fragments	
S-2	2.0	6	3-2-3-3	5		-			Same	
S-3	4.0	15	3-7-6-5	13	-6.0	5-			Gray, moist, stiff, sandy silt FILL	
S-4	6.0	20	5-7-8-10	15	-6.0	-	ML		Brown and gray, mottled, moist, stiff, Sandy SILT	
S-5	8.0	17	1-1-2-3	3		10 –			Gray, moist, micaceous, soft, Sandy SILT	
					-13.0	- -	SM	- 100 Z	Brown and gray, moist, loose, Silty SAND with gravel	
S-6	13.0	12	2-3-6-13	9		15 –				<u>▼/</u>
					-18.0	- -				<u>₹</u> -
S-7	18.0	16	3-3-5-7	8	. 5.5	20 -	ML		Orange and brown, wet, medium stiff, Sandy SILT	
						-				
S-8	23.0	16	5-9-16-21	25	-23.0	-	SM		Brown, wet, medium dense, micaceous, Silty SAND, contains rock fragments	
		_			-25.0	25 -		4 1.4	Boring terminated at 25.0 feet.	
						-				

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

LOG OF BORING NO. B-5

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft):

¥ 8.0

⊈ Dry 17.2

▼ BOC 4/29/16 4/29/16 4/29/16

PROJECT NO.: 160621

PROJECT LOCATION: New Castle County, Delaware

DATE: _

CAVED (ft): In Auger

WATER ENCOUNTERED DURING DRILLING (ft) 8.0

DATE COMPLETED: 4/29/2016

DATE STARTED: 4/29/2016

DRILLING CONTRACTOR: GTA

GROUND SURFACE ELEVATION:

DRILLER: D. Hans

DATUM: Topo

EQUIPMENT: CME550X

LOGGED BY: T. Hill

DRILLING METHOD: Hollow Stem Auger SAMPLING METHOD: Split Spoon

CHECKED BY: C. Reith

SAN	IPLIN	J WIE II	HOD: Spli	t Spo	on				CHECKED BY:	C. Keitii
SAMPLE NUMBER	SAMPLE DEPTH (ft.)	SAMPLE RECOVERY (in.)	SAMPLE BLOWS/6 inches	N (blows/ft.)	ELEVATION (ft.)	DEPTH (ft.)	nscs	GRAPHIC SYMBOL	DESCRIPTION	REMARKS
S-1	0.0	5	1-1-1-1	2	0.0 -0.1	0-	SM		\Topsoil +/- 1 inches Dark brown, moist, very loose, micaceous, Silty SAND	-
S-2	2.0	10	1-1-1-1	2		-			Same	
S-3	4.0	12	1-1-2-3	3		5 –			Same	
S-4	6.0	1 1	4-5-8-4	13		-			Same, medium dense	▼
S-5	8.0	14	4-18-20-21	38		- 10 -			Orange, wet, dense, Silty SAND, contains gravel	=
						-				
S-6	13.0	2	4-10-14-10	24		15 –			Brown, wet, medium dense, micaceous, Silty SAND	
						-				
S-7	18.0	16	3-4-4-5	8		-			Brown, orange and black, wet, loose, micaceous, Silty SAND	
						20 - -				
						-			Same	
S-8	23.0	16	2-2-3-4	5		25 -				
						-				
S-9	28.0	18	2-3-4-6	7	-28.0	-	ML		Brown, orange and gray, wet, medium stiff, micaceous, Sandy SILT	

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

LOG OF BORING NO. B-6

PROJECT:

NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft): \$\frac{\firet{\fracc}{\frac}}}}{\fracc}{\frac{\frac{\frac{\frac{\frac{\frac{\frac{\frac{\frac{\fracc}\

₩ BOC 4/29/16

PROJECT NO.: 160621

PROJE	CTLC	CATIO	N: New	/ Cast	le Cou	nty, [)elav	vare	CAVED (ft): In Auger 17.2	
SAMPLE NUMBER	SAMPLE DEPTH (ft.)	(1	SAMPLE BLOWS/6 inches	N (blows/ft.)	ELEVATION (ft.)	DEPTH (ft.)		GRAPHIC SYMBOL	DESCRIPTION REMARKS	S
					22.0	30 –				
S-10	33.0	13	2-6-8-8	14	-33.0	35 -	SM		Orange, black and brown, wet, medium dense, micaceous, Silty SAND	
S-11	35.0	10	10-10-16- 31	26	-37.0	3 5]			Same, medium dense	
					-37.0	40			Boring terminated at 37.0 feet.	
						45 				
						50 - -				
						55 -				
						60 -				
						65 -				



GEO-TECHNOLOGY ASSOCIATES, INC.

LOG OF BORING NO. B-6

18 Boulden Circle, Suite 36 New Castle, DE 19720

Sheet 2 of 2

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft):

¥ 8.0 ¥ Dry

PROJECT NO .: 160621

PROJECT LOCATION: New Castle County, Delaware

DATE: _

CAVED (ft): In Auger 6.7

4/25/16 4/25/16

4/26/16 7.0

DATE STARTED: 4/25/2016

WATER ENCOUNTERED DURING DRILLING (ft) 8.0

GROUND SURFACE ELEVATION:

DATUM: Topo

DRILLING CONTRACTOR: GTA

DATE COMPLETED: 4/25/2016

DRILLER: D. Hans

EQUIPMENT: CME550X

DRILLING METHOD: Hollow Stem Auger SAMPLING METHOD: Split Spoon

LOGGED BY: T. Hill CHECKED BY: C. Reith

SAN	1PLIN	G METH	OD: Spli	t Spo	on				CHECKED BY	/: C. Reith
SAMPLE NUMBER	SAMPLE DEPTH (ft.)	SAMPLE RECOVERY (in.)	SAMPLE BLOWS/6 inches	N (blows/ft.)	ELEVATION (ft.)	DEPTH (ft.)	nscs	GRAPHIC SYMBOL	DESCRIPTION	REMARKS
S-1	0.5	19	1-2-2-1	4	0.0 -0.5	0-	sc		Topsoil +/- 6 inches Brown, moist, very loose, Clayey SAND	
S-2	2.0	16	1-2-6-6	8	-2.0	_	CL		Brown, moist, medium stiff,Sandy Lean CLAY	
S-3	4.0	8	1-2-2-3	4		5 -			Same, soft	
S-4	6.0	7	19-50/5	50/5	-6.0	-	HW	Δ.Δ Δ.Δ Δ.Δ Δ.Δ	Brown, white, moist, very dense, Highly Weathered ROCK	Auger grinding 6.0
S-5	8.0	18	21-30-16- 18	46		-		12 4 4 4 4 4 4 4 4 4 4	Same, brown, gray, wet	to 8.0 feet.
					-10.0	10 -		R: A: A	Boring terminated at 10.0 feet.	
						- - 15 – -				
						20 - -				
						25 — -				
						-				

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

LOG OF BORING NO. B-7

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft): 7.5

4/26/16

PROJECT LOCATION: New Castle County, Delaware

PROJECT NO.: 160621

DATE: _ CAVED (ft): In Auger

4/25/16 4/25/16

7.6

DATE STARTED: 4/25/2016

WATER ENCOUNTERED DURING DRILLING (ft) 7.5

GROUND SURFACE ELEVATION:

DATE COMPLETED: 4/25/2016 DRILLING CONTRACTOR: GTA

DATUM: Topo

DRILLER: D. Hans

EQUIPMENT: CME550X

LOGGED BY: T. HIII

DRILLING METHOD: Hollow Stem Auger SAMPLING METHOD: Split Spoon

	CHECKED BY:	C. Reith
--	-------------	----------

SAM	IPLIN	GMETH	HOD: Spli t	t Spo	on				CHECKED BY	C. Reiui
SAMPLE NUMBER	SAMPLE DEPTH (ft.)	SAMPLE RECOVERY (in.)	SAMPLE BLOWS/6 inches	N (blows/ft.)	ELEVATION (ft.)	DEPTH (ft.)	nscs	GRAPHIC SYMBOL	DESCRIPTION	REMARKS
S-1	0.0	16	1-1-1-2	2	0.0 -0.7	0-	ML	1, <u>00</u> 44	Topsoil +/- 8 inches Brown, moist, very soft, Sandy SILT	
S-2	2.0	12	1-1-1-2	2	-2.0	- -	SM		Brown, moist, very loose, Silty SAND	
S-3	4.0	13	1-1-2-2	3		5-			Same	
S-4	6.0	14	6-11-30-25	41		-			Brown, moist, dense, Silty SAND with rock fragments	Access spinding 7.0
S-5	8.0	10	4-7-18-27	25	-10.0	10			Same, brown and gray, wet, medium dense Boring terminated at 10.0 feet.	Auger grinding 7.0 to 8.0 feet.
						15 20 25				

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

LOG OF BORING NO. B-8

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft): ¥ 8.0 DATE: -

T Dry

▼ Dry

PROJECT NO.: 160621 PROJECT LOCATION: New Castle County, Delaware

4/25/16 CAVED (ft): In Auger

6.2

4/25/16 4/26/16 5.8

DATE STARTED: 4/25/2016

WATER ENCOUNTERED DURING DRILLING (ft) 8.0

GROUND SURFACE ELEVATION:

DATE COMPLETED: 4/25/2016

DATUM: Topo

DRILLING CONTRACTOR: GTA

EQUIPMENT: CME550X

DRILLER: D. Hans

DRILLING METHOD: Hollow Stem Auger

LOGGED BY: T. Hill CHECKED BY: C. Reith

SAIV	IPLIN	GMETH	OD: Split	t Spoo	on				CHECKED B	Y: C. Reith
SAMPLE	SAMPLE DEPTH (ft.)	SAMPLE RECOVERY (in.)	SAMPLE BLOWS/6 inches	N (blows/ft.)	ELEVATION (ft.)	DEPTH (ft.)	SOSN	GRAPHIC SYMBOL	DESCRIPTION	REMARKS
S-1	0.0	18	1-3-2-3	5	0.0 -0.4	0-	sc		Topsoil +/- 5 inches Brown, moist, loose, Clayey SAND	
S-2	2.0	20	2-1-1-1	2	-2.0	-	SM		Brown, moist, very loose, Silty SAND	
S-3	4.0	14	2-7-16-17	23		5 –			Brown, white, moist, medium dense, Silty SAND, contains rock fragments	
S-4	6.0	8	24-36-50/2	86/8	-6.0	_	HW	ΔΔ ΔΔ ΔΔ ΔΔ	Brown, moist, very dense, Highly Weathered ROCK	<u>▼</u>
S-5	8.0	16	15-25-31- 23	56	-10.0	10		44 44 44	Same, wet	-
					-10.0	-			Boring terminated at 10.0 feet.	
						15 —				
						20 -				

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

LOG OF BORING NO. B-9

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft):

¥ 8.0

¥ 7.5

5.8

PROJECT NO.: 160621

PROJECT LOCATION: New Castle County, Delaware

DATE: -

4/25/16 CAVED (ft): In Auger

4/25/16 8.0

4/26/16 7.6

DATE STARTED: 4/25/2016

WATER ENCOUNTERED DURING DRILLING (ft) 8.0 **GROUND SURFACE ELEVATION:**

DATE COMPLETED: 4/25/2016

DATUM: Topo

DRILLING CONTRACTOR: GTA

DRILLER: D. Hans

EQUIPMENT: CME550X

LOGGED BY: T. Hill

DRILLING METHOD: Hollow Stem Auger SAMPLING METHOD: Split Spoon

CHECKED BY: C. Reith

SAIV	LL	G MIL II	iou: Spii i	- Cpo	/ 11		_	_		. 0.110101
SAMPLE NUMBER	SAMPLE DEPTH (ft.)	SAMPLE RECOVERY (in.)	SAMPLE BLOWS/6 inches	N (blows/ft.)	ELEVATION (ft.)	DЕРТН (Æ.)	nscs	GRAPHIC SYMBOL	DESCRIPTION	REMARKS
S-1	0.0	10	1-3-2-2	5	0.0 -0.3 -2.0	0	ML		Topsoil +/- 4 inches Brown, moist, medium stiff, Sandy SILT	_
S-2	2.0	12	2-2-2-3	4	2.0	_	SM		Brown, moist, very loose, Silty SAND	
S-3	4.0	10	2-2-3-4	5		5			Same, loose, contains rock fragments	▼
S-4	6.0	10	9-18-14-10	32					Brown, white, moist, dense, Silty SAND, contains rock fragments	
3-4	0.0		9-10-14-10	J2	-8.0	-			-	<u>▼</u>
S-5	8.0	12	6-20-38-10	58	-10.0	10 -	HW	A A A A	Brown, wet, very dense, Highly Weathered ROCK Boring terminated at 10.0 feet.	
						15 – 20 – 25 –				

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

LOG OF BORING NO. B-10

PROJECT: NVF Flood Plain Mitigation & Culverts

WATER LEVEL (ft):

PROJECT NO.: 160621

PROJECT LOCATION: New Castle County, Delaware

DATE: _ CAVED (ft): In Auger

¥ 6.0

4/25/16 4/25/16 8.0

4/26/16 6.0

DATE STARTED: 4/25/2016

WATER ENCOUNTERED DURING DRILLING (ft) 6.0

GROUND SURFACE ELEVATION:

DRILLING CONTRACTOR: GTA

DATE COMPLETED: 4/25/2016

DATUM: Topo

DRILLER: D. Hans

EQUIPMENT: CME550X

LOGGED BY: T. Hill

DRILLING METHOD: Hollow Stem Auger SAMPLING METHOD: Split Spoon

CHECKED BY: C. Reith

O/NIV	IF LIN	GMETT	IOD: Spin	. opo	ווע				OTILOTES :	or Reini
SAMPLE	SAMPLE DEPTH (ft.)	SAMPLE RECOVERY (in.)	SAMPLE BLOWS/8 inches	N (blows/ft.)	ELEVATION (ft.)	DЕРТН (ft.)	nscs	GRAPHIC SYMBOL	DESCRIPTION	REMARKS
							-			
S-1	0.0	13	1-1-1-1	2	0.0 -0.4 -2.0	0-	ML		Topsoil +/- 5 inches Brown, moist, soft, Sandy SILT	
S-2	2.0	21	1-1-2-2	3		-	sc		Brown, moist, very loose, Clayey SAND	<u> </u>
S-3	4.0	14	1-2-3-4	5	-4.0	5-	SM		Brown, moist, loose, Silty SAND	<u> </u>
S-4	6.0	10	7-10-15-15	25		-			Brown and white, wet, medium dense, Silty SAND, contains rock fragments	V
S-5	8.0	16	7-11-16-17	27	-10.0	10 -			Same	
					-10.0	-		;	Boring terminated at 10.0 feet.	
						- 15 — -				
						20 -				
						25 -				
						-				

NOTES:



GEO-TECHNOLOGY ASSOCIATES, INC.

18 Boulden Circle, Suite 36 New Castle, DE 19720

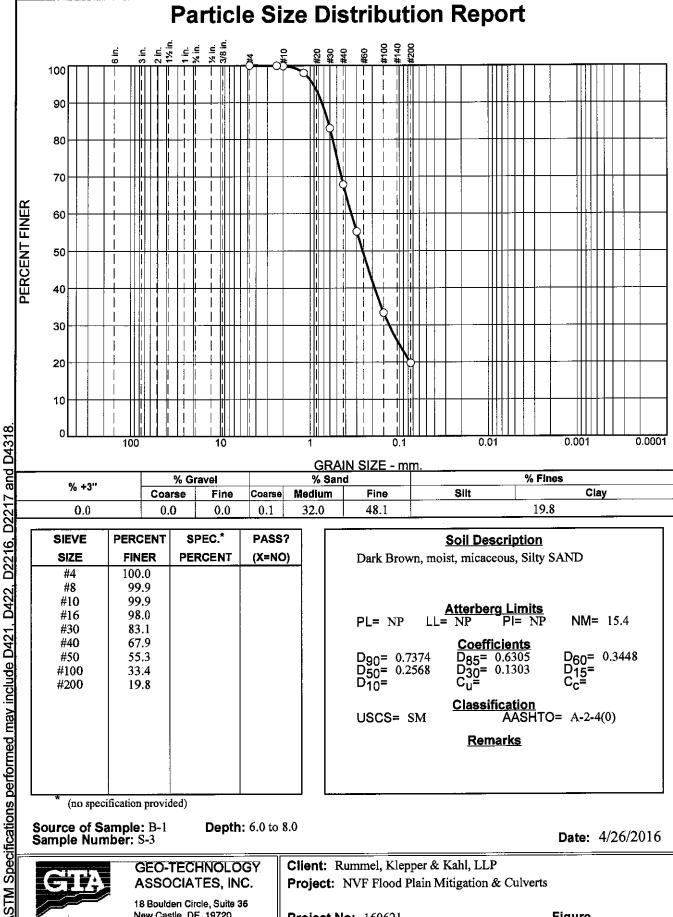
LOG OF BORING NO. B-11

APPENDIX C

CONTRACT DATE	160621 6/2/2016							Ż	NAME N	NVF F10	od Pla	Flood Plain Mitigation	igatio	ъ	Culverts
-				PE	PERCENT PASSING	ASSING				-	-		-	į	0 4
LOCATION	DEPTH	2.5	2	1	3/8	No. 4	No. 10	No. 40	No. 200	T.L	2	OM M	ž	Ξ	CLASS
B-1	0.0-2.0	100.0	100.0	100.0	100.0	100.0	100.0	92.2	44.3	32	21	21.0	• • •	11	A-6(2)
	2.0-4.0	100.0	100.0	100.0	100.0	100.0	99.9	67.9	19.8	NP	NP	7.7		NP	A-2-4(0)
B-2						<u> </u>									
	4.0-6.0 8.0-10.0 13.0-15.0	100.0	100.0	100.0	100.0	93.0	88.0	61.0	19.0	NP	NP	25.9 11.5		NP	A-2-4(0)
B-3															
	2.0-4.0											15.4 25.6			
	13.0-15.0							-				19.3			
B-4		,	100	_	100	1	100	60	44.3	33	-12	21			A-6(2)
	2.0-4.0	100.0	100.0		100.0	89.2	83.6	64.6	31.8	27	19	11.6		8	A-2-4(0)
	6.0-8.0	100.0	100.0	0.00	100.0	97.4	93.2	0.69	21.9	NP	NP	15.7 3.3		NP	A-2-4(0)
B-5												[
	2.0-4.0 8.0-10.0	100.0	100.0	100.0	100.0	99.3	96.3	84.5	63.9	32	27	25.5		2	A-4 (2)
	13.0-15.0 18.0-20.0	100.0	100.0	100.0	100.0	96.9	93.3	83.4	58.0	42	28	11.1		14	A-7-6(7)
B-6	0									i		7			
	6.0-8.0	100.0	100.0	100.0	100.0	99.5	0.66	89.6	18.6	NP	NP	12.3		NP	A-2-4(0)
	13.0-15.0 23.0-25.0 33.0-35.0	100.0	100.0	100.0	100.0	98.5	92.9	73.6	42.9	49	33	28.0		16 9	A-7-5(4) A-4(1)
B-7				0			100	92.2	44.3	3.5	2.	0 10		-	A-6(2)
	2.0-4.0	100.0		100.0	100.0		0.66	94.2	57.1	36	24	25.5		12	A-6(5)
B-8	2.0-4.0	100.0	100.0	100.0	100.0	99.3	98.9	91.7	36.9			20.4			ļ
B-9	0.4-0.4	100.0	100.0	100.0	100.0	100.0	100.0	92.2	44.3	32	21	21.0		11	A-6(2)
B-10												-			
B-11															
															ļ

PAGE 1

SUMMARY OF SOIL ANALYSIS TESTS



GRAIN SIZE - mm. % Fines % Gravel % +3" Coarse Medium Fine Silt Clay Coarse Fine 19.8 0.0 32.0 48.1 0.0

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
#4	100.0		
#8	99.9		
#10	99.9		
#16	98.0		
#30	83.1		
#40	67.9		
#50	55.3		
#100	33.4		
#200	19.8		

Dark Brown, moi	Soil Description st, micaceous, Sil	
PL= NP LL=	Atterberg Limit	<u>ts</u> NP NM= 15.4
D ₉₀ = 0.7374 D ₅₀ = 0.2568 D ₁₀ =	Coefficients D85= 0.6305 D30= 0.1303 Cu=	D ₆₀ = 0.3448 D ₁₅ = C _c =
USCS= SM		1 HTO= A-2-4(0)
	<u>Remarks</u>	
D ₁₀ =	D ₃₀ = 0.1303 C _u = Classification	1

(no specification provided)

Source of Sample: B-1 Sample Number: S-3

Depth: 6.0 to 8.0

Date: 4/26/2016



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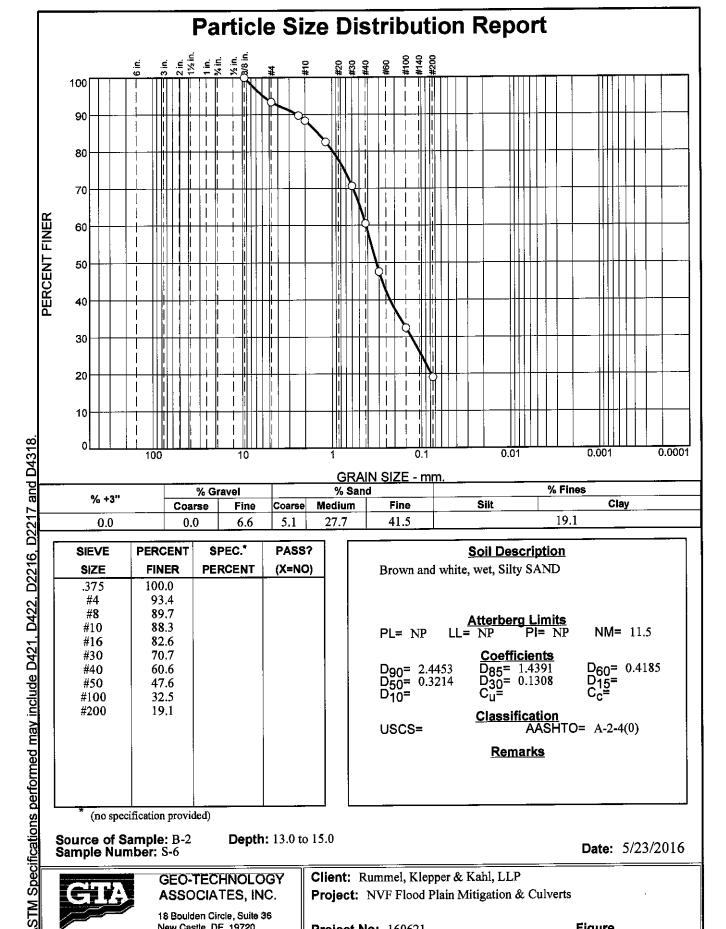
Client: Rummel, Klepper & Kahl, LLP

Project: NVF Flood Plain Mitigation & Culverts

Project No: 160621

Figure

Tested By: M. Emery



SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
.375	100.0		
#4	93.4		
#8	89.7		
#10	88.3		
#16	82.6		
#30	70.7		
#40	60.6		
#50	47.6		
#100	32.5		
#200	19.1		

Coarse

0.0

	Soil Description , wet, Silty SAND	
PL= NP LL=	Atterberg Limits NP PI= NI	P NM= 11.5
D ₉₀ = 2.4453 D ₅₀ = 0.3214 D ₁₀ =	Coefficients D85= 1.4391 D30= 0.1308 Cu=	D ₆₀ = 0.4185 D ₁₅ = C _c =
USCS=	Classification AASHT	ΓO= A-2-4(0)
	Remarks	

Silt

(no specification provided)

Source of Sample: B-2 Sample Number: S-6

% +3"

0.0

Depth: 13.0 to 15.0

Coarse

Fine

6.6

Medium

27.7

Fine

41.5

Date: 5/23/2016

Clay

19.1



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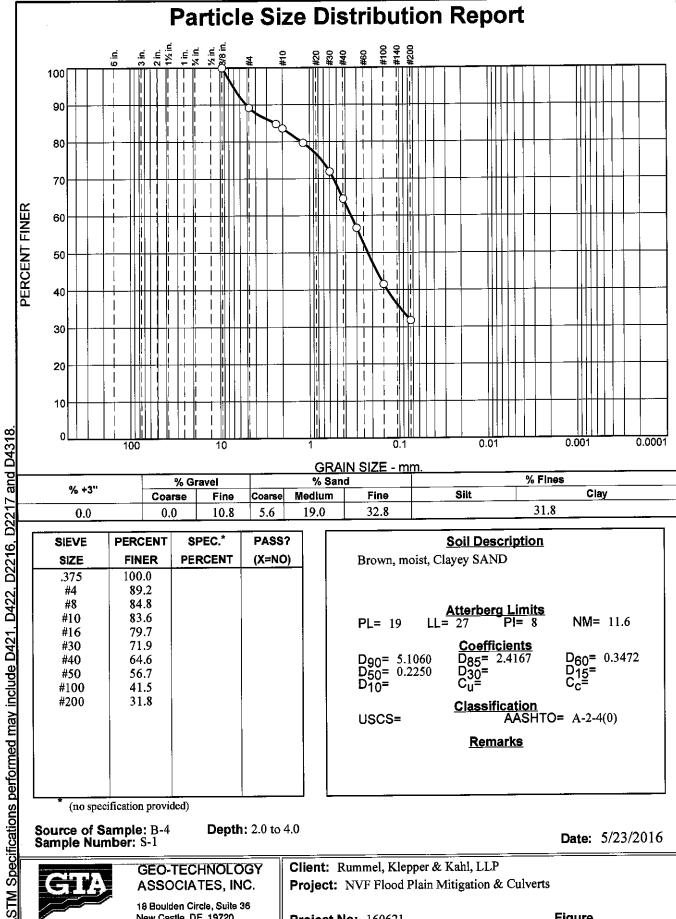
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Project No: 160621

Figure

Tested By: M. Sobus



GRAIN SIZE - mm % Fines % Gravel % Sand % +3" Şilt Clay Medium Fine Coarse Fine Coarse 31.8 5.6 0.0 0.0 10.8 19.0 32.8

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X≓NO)
.375	100.0		
#4	89.2		
#8	84.8		
#10	83.6		
#16	79.7		
#30	71.9		
#40	64.6		
#50	56.7		
#100	41.5		
#200	31.8		

Brown, moist, C	Soil Description	
Diown, moist, O.	(a) 0) 2111.2	
PL= 19 LL:	Atterberg Limits = 27 PI= 8	NM= 11.6
D ₉₀ = 5.1060 D ₅₀ = 0.2250 D ₁₀ =	<u>Coefficients</u> D ₈₅ = 2.4167 D ₃₀ = C _u =	D ₆₀ = 0.3472 D ₁₅ = C _c =
USCS=	<u>Classification</u> AASHT	O= A-2-4(0)
	<u>Remarks</u>	
		:

(no specification provided)

Source of Sample: B-4 Sample Number: S-1

Depth: 2.0 to 4.0

Date: 5/23/2016



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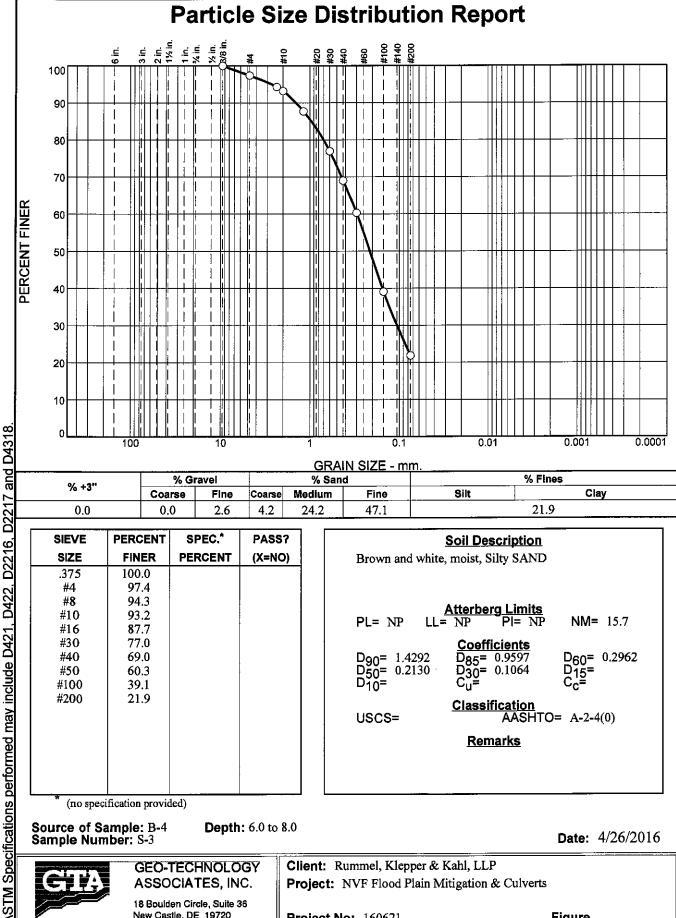
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Project No: 160621

Figure

Tested By: M. Sobus



				GRA	IIN SIZE - MI	m		
0/ .00	% Gı	ravel		% San	d		% Fines	
% +3"	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay	
0.0	0.0	2.6	4.2	24.2	47.1		21.9	

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
.375	100.0		
#4	97.4		
#8	94.3		
#10	93.2		
#16	87.7		
#30	77.0		
#40	69.0		
#50	60.3		
#100	39.1		
#200	21.9		

	Soil Description , moist, Silty SAND	
PL= NP LL=	Atterberg Limits NP PI= NP	NM= 15.7
D ₉₀ = 1.4292 D ₅₀ = 0.2130 D ₁₀ =	Coefficients D85= 0.9597 D30= 0.1064 Cu=	D ₆₀ = 0.2962 D ₁₅ = C _c =
USCS=	Classification AASHT	O= A-2-4(0)
	<u>Remarks</u>	

(no specification provided)

Source of Sample: B-4 **Sample Number:** S-3

Depth: 6.0 to 8.0

Date: 4/26/2016



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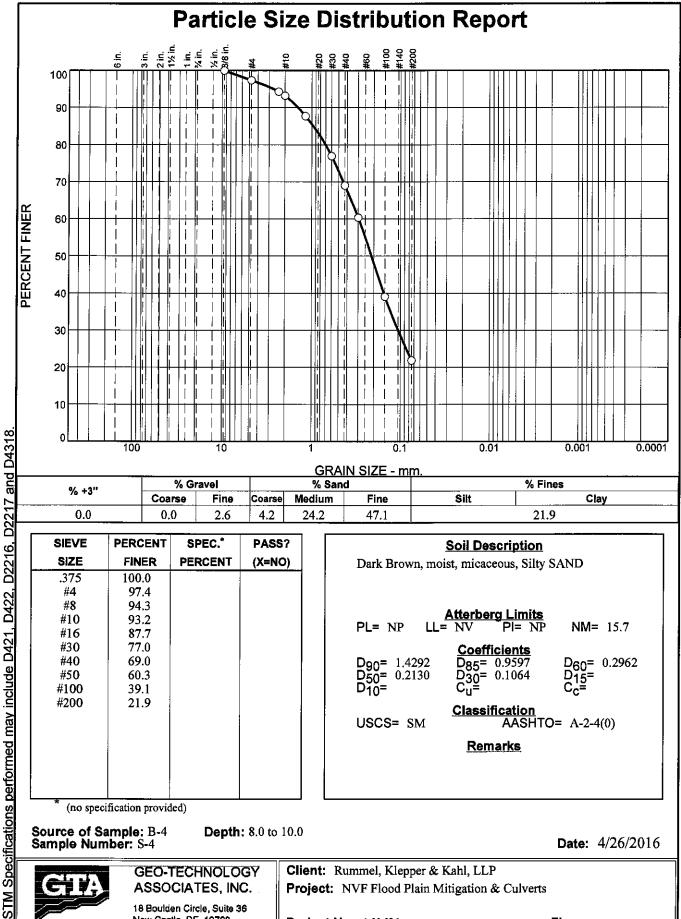
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Project: NVF Flood Plain Mitigation & Culverts

Project No: 160621

Figure

Tested By: M. Emery



				UKA	<u> IIV SIZE - IIII</u>	II.		
% +3"	% Gr	avel		% San	d		% Fines	
76 T3	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay	
0.0	0.0	2.6	4.2	24.2	47.1		21.9	

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
.375	100.0		
#4	97.4		
#8	94.3		
#10	93.2		
#16	87.7		
#30	77.0		
#40	69.0		:
#50	60.3		
#100	39.1		
#200	21.9		

Soil Des	<u>scription</u>
Dark Brown, moist, micace	eous, Silty SAND

Atterberg Limits
LL= NV PI= NP PL= NP NM= 15.7

Coefficients D₉₀= 1.4292 D₅₀= 0.2130 D₁₀= D₈₅= 0.9597 D₃₀= 0.1064 C_u= D₆₀= 0.2962

Classification AASHTO= A-2-4(0) USCS= SM

Remarks

(no specification provided)

Source of Sample: B-4 Sample Number: S-4

Depth: 8.0 to 10.0

Date: 4/26/2016



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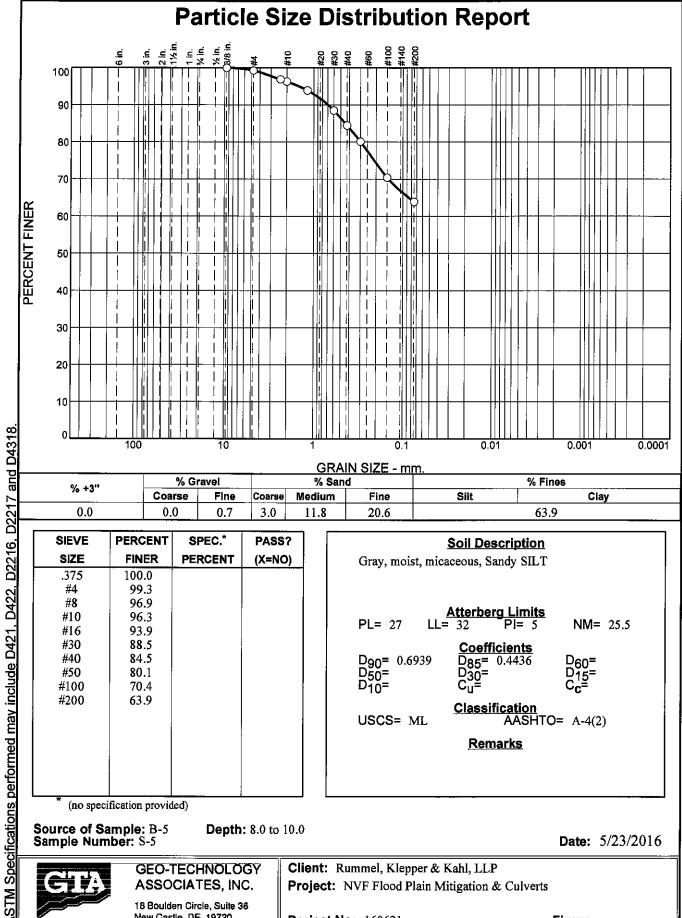
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Project: NVF Flood Plain Mitigation & Culverts

Project No: 160621

Figure

Tested By: M. Emery



				GNA	<u> </u>	U.		
% +3"	% Gr	avel		% San	D.		% Fines	
76 TJ	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay	
0.0	0.0	0.7	3.0	11.8	20.6		63.9	

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
.375	100.0		
#4	99.3		
#8	96.9		
#10	96.3		
#16	93.9		
#30	88.5		
#40	84.5		
#50	80.1		
#100	70.4		
#200	63.9		

Gray, moist, micaceous, Sandy SILT Atterberg Limits PL= 27	Atterberg Limits PL= 27		Soil Description	<u>on</u>
PL= 27	Coefficients Description Coefficients Description Classification AASHTO= A-4(2)	Gray, moist, mic	caceous, Sandy SII	LT
PL= 27	Coefficients Description Coefficients Description Coefficients Description Description Description Classification AASHTO= A-4(2)			
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	D ₉₀ = 0.6939	PL= 27 LL	Atterberg Limi = 32 PI= 3	<u>ts</u> 5 NM= 25.5
USCS= ML Cu= Classification AASHTO= A-4(2)	Classification JSCS= ML AASHTO= A-4(2)			
USCS= ML AASHTO= A-4(2)	JSCS= ML AASHTO= A-4(2)	550 - 510=	C ⁿ =	$C_{\mathbf{c}}^{15}$
	` ,	ISCS- M	Classification	1
remarks	remarks	JOCO- MIL		110- A-4(2)
			Remarks	

(no specification provided)

Source of Sample: B-5 **Sample Number:** S-5

Depth: 8.0 to 10.0

Date: 5/23/2016



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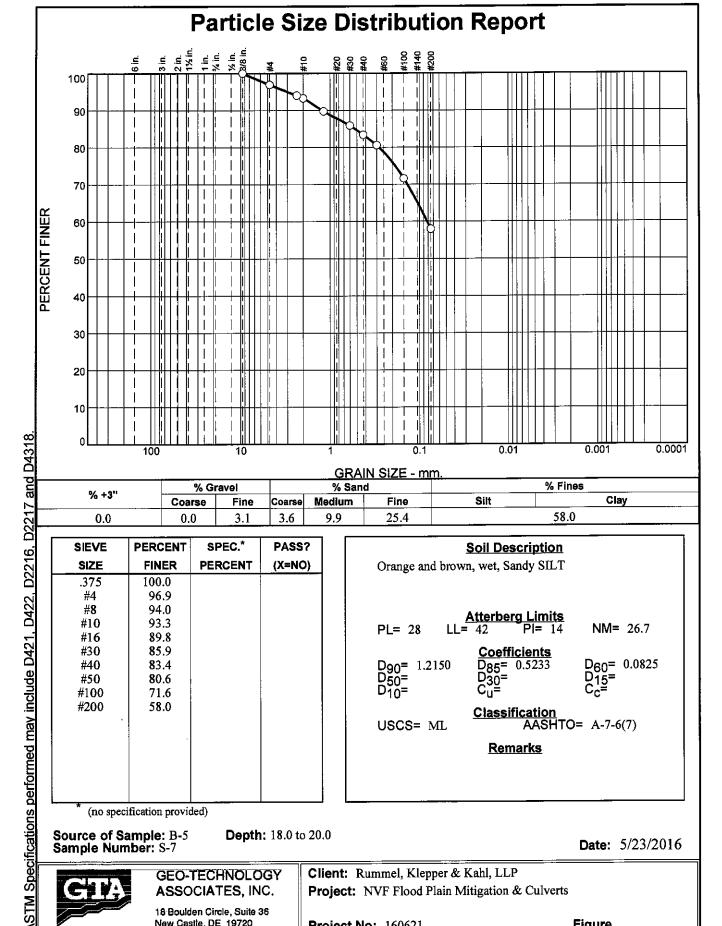
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Project: NVF Flood Plain Mitigation & Culverts

Project No: 160621

Figure

Tested By: M. Sobus



_	0.0		0.	0	3.1	3.6	
[CIEVE	PERC	ENT		PEC.*	PASS	2
١	SIEVE	PERC	, ENI	3	PEG.	PAJJ	16
l	SIZE	FIN	ER	PE	RCENT	(X=NC)
	.375	100	.0				
١	#4	96	.9				
١	#8	94	.0				
١	#10	93	.3				
١	#16	89	.8				
١	#30	85	.9				
١	#40	83	.4				
١	#50	80	.6				
١	#100	71	.6				
١	#200	58	.0				
١							
١							
1							
ŀ							

Coarse

Fine

Coarse

Medium

9.9

Fine

25.4

Soil Description							
Orange and brow	n, wet, Sandy SILT						
PL= 28 LL=	Atterberg Limits 42 Pl= 14	NM= 26.7					
FL- 20 LL-		14141- 20.7					
D ₉₀ = 1.2150	Coefficients D ₈₅ = 0.5233	D ₆₀ = 0.0825					
D50= D10=	D30= Cu=	D15= C _C =					
10	Classification	·					
USCS= ML	AASHT	O= A-7-6(7)					
	<u>Remarks</u>						
		·					

Silt

(no specification provided)

Source of Sample: B-5 **Sample Number:** S-7

% +3"

Depth: 18.0 to 20.0

Date: 5/23/2016

Clay

58.0



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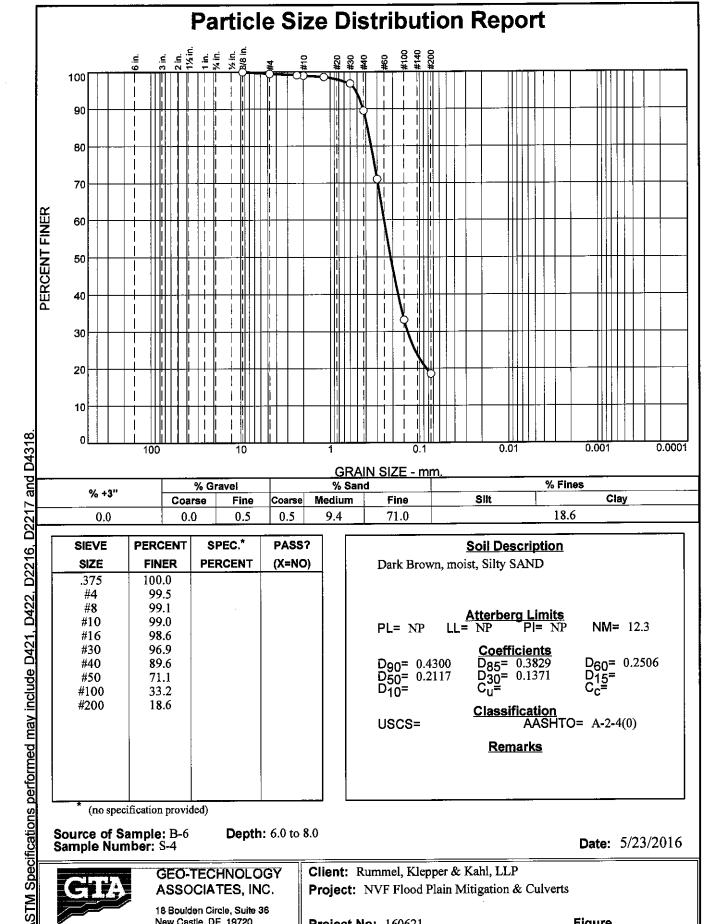
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Project No: 160621

Figure

Tested By: M. Sobus



Medium

9.4

Coarse

0.5

Fine

71.0

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
.375	100.0		
#4	99.5		
#8	99.1		
#10	99.0		
#16	98.6		
#30	96.9		
#40	89.6		
#50	71.1		
#100	33.2		
#200	18.6		
	ĺ		

Coarse

0.0

Fine

0.5

Soil Description Dark Brown, moist, Silty SAND								
PL= NP LL	Atterberg Limi	its NP NM= 12.3						
D ₉₀ = 0.4300 D ₅₀ = 0.2117 D ₁₀ =	Coefficients D85= 0.3829 D30= 0.1371 Cu=	D ₆₀ = 0.2506 D ₁₅ = C _c =						
USCS=	USCS= Classification AASHTO= A-2-4(0)							
<u>Remarks</u>								

Silt

(no specification provided)

Source of Sample: B-6 **Sample Number:** S-4

0.0

Depth: 6.0 to 8.0

Date: 5/23/2016

Clay

18.6



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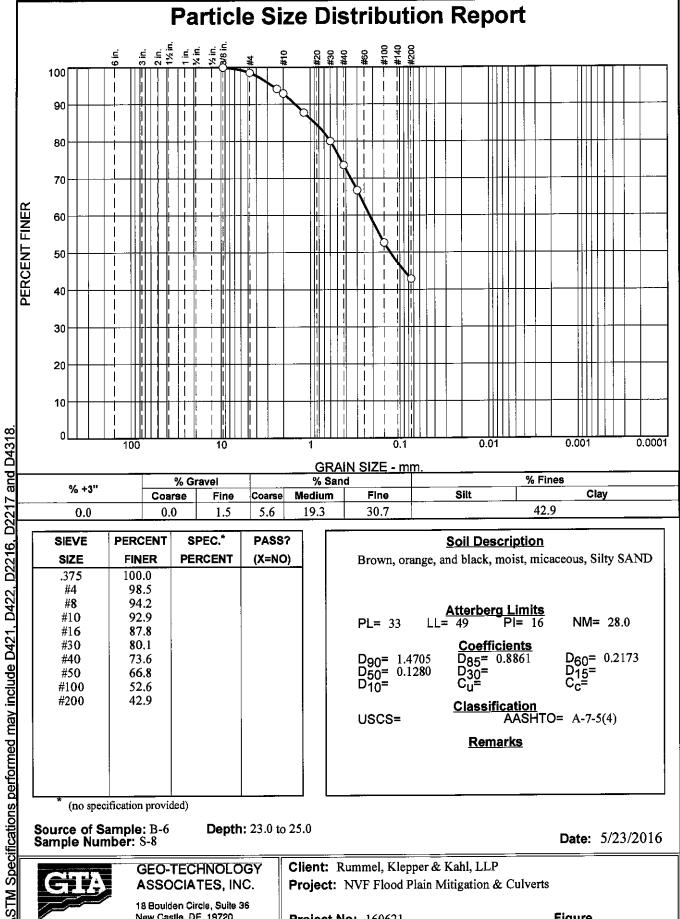
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Project: NVF Flood Plain Mitigation & Culverts

Project No: 160621

Figure

Tested By: M. Sobus



	OIVAIT OIZE - IIIIII.								
	% Gr	avel	% Sand		d	% Fines			
% +3"	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay		
0.0	0.0	1.5	5.6	19.3	30.7		42.9		

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
.375	100.0		
#4	98.5		
#8	94.2		
#10	92.9		
#16	87.8		
#30	80.1		
#40	73.6		
#50	66.8		
#100	52.6		
#200	42.9		
1			

Soil Description Brown, orange, and black, moist, micaceous, Silty SAND							
PL= 33 LL	Atterberg Limits = 49 PI= 16	NM= 28.0					
D ₉₀ = 1.4705 D ₅₀ = 0.1280 D ₁₀ =	Coefficients D85= 0.8861 D30= Cu=	D ₆₀ = 0.2173 D ₁₅ = C _c =					
USCS=	USCS= Classification AASHTO= A-7-5(4)						
<u>Remarks</u>							

(no specification provided)

Source of Sample: B-6 Sample Number: S-8

Depth: 23.0 to 25.0

Date: 5/23/2016



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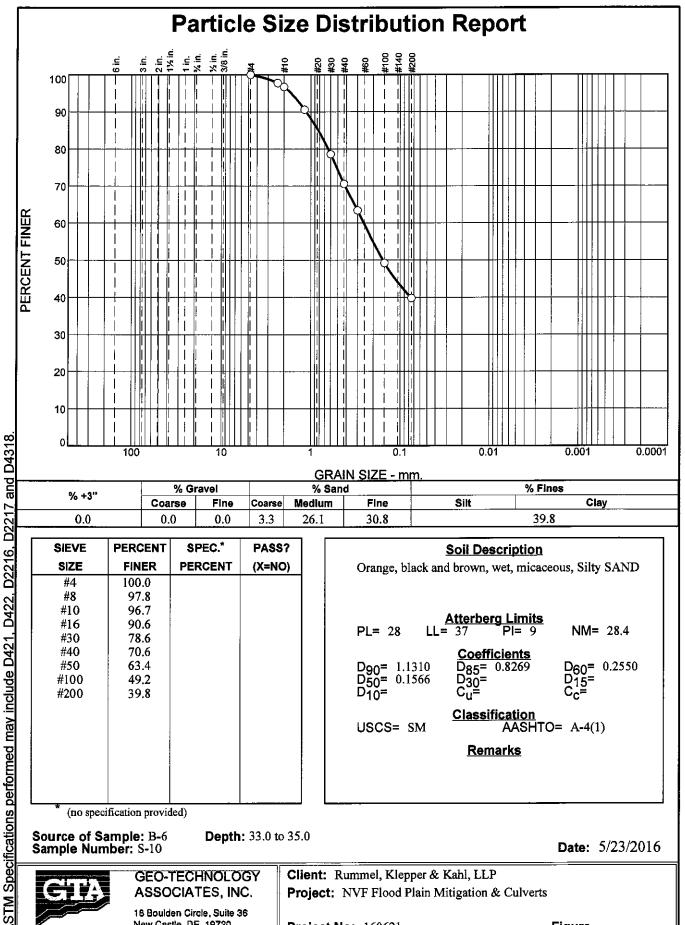
Client: Rummel, Klepper & Kahl, LLP

Project: NVF Flood Plain Mitigation & Culverts

Project No: 160621

Figure

Tested By: M. Sobus



% Gravel % Fines % Sand % +3" Silt Clay Medium Fine Coarse Fine Coarse 0.0 0.0 0.0 3.3 26.1 30.8 39.8

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
#4	100.0		
#8	97.8		
#10	96.7		
#16	90.6		
#30	78.6		
#40	70.6		
#50	63.4		
#100	49.2		
#200	39.8		

Soil Description								
Orange, black and brown, wet, micaceous, Silty SAND								
Atterberg Limits								
PL= 28 LL=	= 37 PI= 9	NM= 28.4						
D ₉₀ = 1.1310 D ₅₀ = 0.1566 D ₁₀ =	Coefficients D ₈₅ = 0.8269 D ₃₀ = C _u =	D ₆₀ = 0.2550 D ₁₅ = C _c =						
USCS= SM	USCS= SM Classification AASHTO= A-4(1)							
	Remarks							
								

(no specification provided)

Source of Sample: B-6 Sample Number: S-10

Depth: 33.0 to 35.0

Date: 5/23/2016



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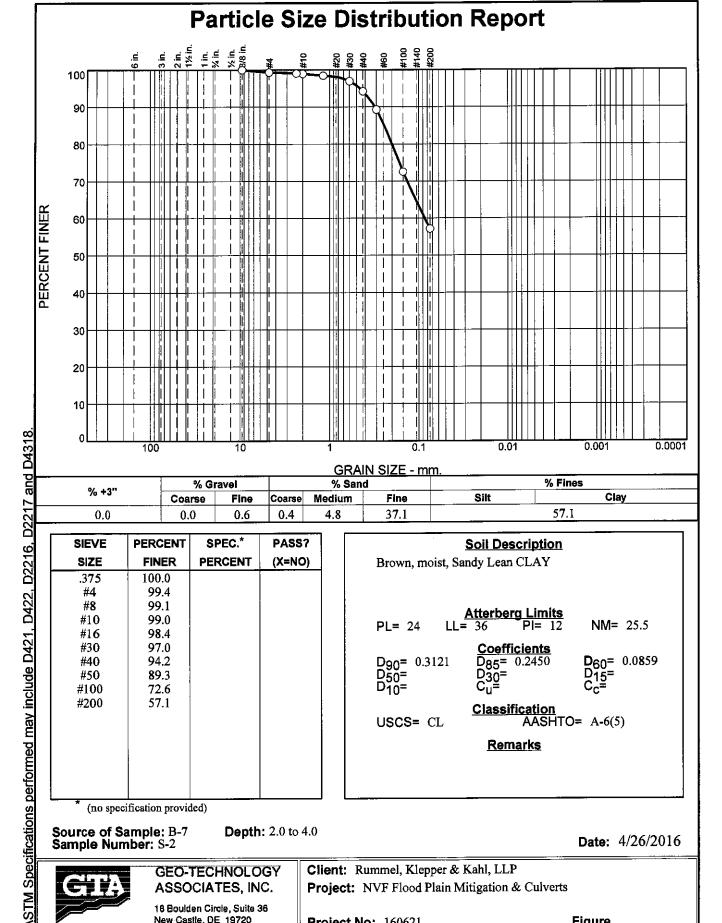
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Project: NVF Flood Plain Mitigation & Culverts

Project No: 160621

Figure

Tested By: M. Sobus



SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
.375	100.0		
#4	99.4		
#8	99.1		İ
#10	99.0		
#16	98.4		
#30	97.0		
#40	94.2		
#50	89.3		
#100	72.6		
#200	57.1		
* .	12 1		

Coarse

0.0

Fine

0.6

Coarse

0.4

Medium

4.8

Fine

37.1

Soil Description Brown, moist, Sandy Lean CLAY						
PL= 24 LL=	Atterberg Limi = 36 PI=					
D ₉₀ = 0.3121 D ₅₀ = D ₁₀ =	Coefficients D85= 0.2450 D30= Cu=					
USCS= CL	<u>Classification</u>					
<u>Remarks</u>						

Silt

* (no specification provided)

Source of Sample: B-7 **Sample Number:** S-2

% +3"

0.0

Depth: 2.0 to 4.0

Date: 4/26/2016

Clay

57.1



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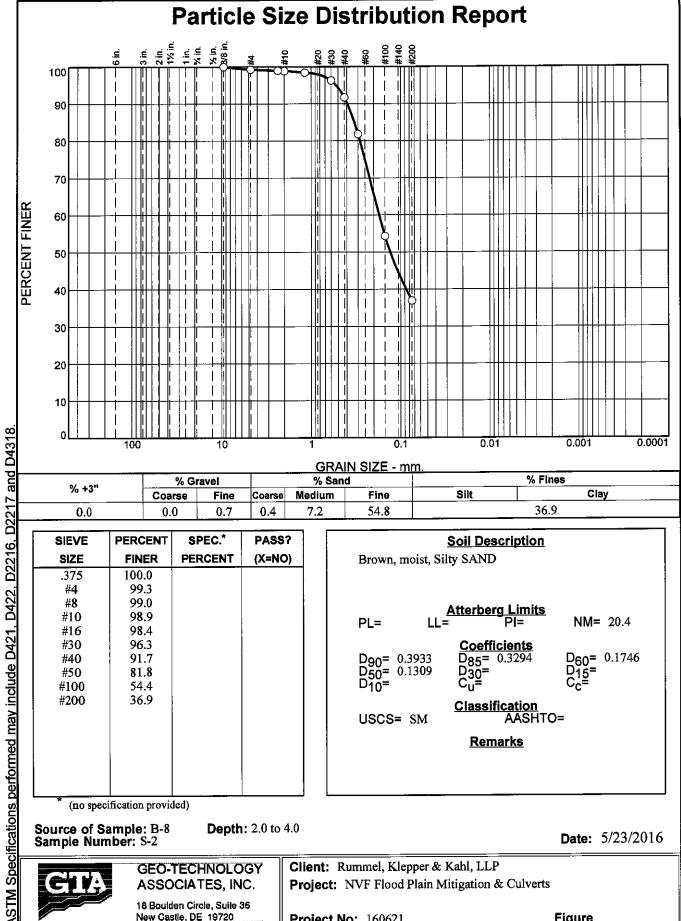
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Project: NVF Flood Plain Mitigation & Culverts

Project No: 160621

Figure .

Tested By: M. Emery



% +3"		% Gr	Gravel % Sand		d	% Fines			
	% +3"	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay	
	0.0	0.0	0.7	0.4	7.2	54.8	36.9		
	•								

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
.375	100.0		
#4	99.3		
#8	99.0		
#10	98.9		:
#16	98.4		
#30	96.3		
#40	91.7		
#50	81.8		
#100	54.4		
#200	36.9		

Soil Description Brown, moist, Silty SAND				
PL= L	<u>Atterberg Limits</u> L= PI=	NM= 20.4		
D ₉₀ = 0.3933 D ₅₀ = 0.1309 D ₁₀ =	Coefficients D ₈₅ = 0.3294 D ₃₀ = C _u =	D ₆₀ = 0.1746 D ₁₅ = C _c =		
USCS= SM AASHTO=				
<u>Remarks</u>				

(no specification provided)

Source of Sample: B-8 **Sample Number:** S-2

Depth: 2.0 to 4.0

Date: 5/23/2016



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Client: Rummel, Klepper & Kahl, LLP

Project: NVF Flood Plain Mitigation & Culverts

Project No: 160621

Figure

Tested By: M. Sobus